

DESIGN CRITERIA

DESIGN PER 2013 CALIFORNIA BUILDING CODE, UNLESS OTHERWISE NOTED.

LIVE LOADS:		
ROOFS AND CANOPIES: (REDUCIBLE)	20 PSF	
DEAD LOADS (SUPERIMPOSED LOAD NOT INCLUDING STRUCTURE SELF WEIGHT):		
FLOOR	N/A	
ROOF	20 PSF	
WIND LOADS:		
ULTIMATE WIND SPEED: (ASCE 7-10)	115 MPH	
NOMINAL WIND SPEED:	90 MPH	
MEAN ROOF HEIGHT:	16 FT	
RISK CATEGORY:	III	
WIND EXPOSURE:	C	
ENCLOSURE CLASSIFICATION:	C	ENCLOSED
INTERNAL PRESSURE COEFFICIENT:	± 0.18	
DIRECTIONALITY FACTOR (K _d):	0.85	
SHAPE FACTORS:		PER CODE

THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGION. IMPACT RESISTANT GLAZING IS NOT REQUIRED.

SNOW LOADS:		
GROUND SNOW LOAD, P _s	0 PSF	
FLAT ROOF SNOW LOAD (INCLUDING RAIN ON SNOW SURCHARGE)	0 PSF	
EXPOSURE FACTOR, C _e	1.0	
THERMAL FACTOR, C _t	1.0	
IMPORTANCE FACTOR, I	1.1	

SEISMIC:		
IMPORTANCE FACTOR, I	1.25	
MAPPED SPECTRAL RESPONSE ACCELERATION, S _a	0.80	
MAPPED SPECTRAL RESPONSE ACCELERATION, S ₁	0.20	
SITE CLASS	D	
SPECTRAL RESPONSE COEFFICIENT, S _{ds}	1.50	
SPECTRAL RESPONSE COEFFICIENT, S _{1s}	0.744	
SEISMIC DESIGN CATEGORY	D	
BASIC STRUCTURAL SYSTEM	BUILDING FRAME SYSTEM	
SEISMIC RESISTING SYSTEM	ORDINARY CONCENTRICALLY BRACED FRAME	
RESPONSE MODIFICATION FACTOR, R	3.25	
ANALYSIS PROCEDURE	EQUIV LATERAL FORCE	
SEISMIC RESPONSE COEFFICIENT, C _s	0.385	
DESIGN BASE SHEAR, V	0.385W	

CONCRETE (DESIGN PER CURRENT EDITION ACI 318)		
SLAB ON GRADE	FC= 3000 PSI	
FOOTINGS	FC= 3000 PSI	
ALL OTHER CONCRETE	FC= 3000 PSI	

ALL REINFORCING STEEL ASTM A615 GRADE 60. REINFORCING STEEL SHOWN ON THESE DRAWINGS TO BE WELDED SHALL BE ASTM A706.

WELDED WIRE FABRIC	ASTM A185
CONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 530)	FM= 1500 PSI
COMPRESSIVE STRENGTH	

STRUCTURAL STEEL (DESIGN PER CURRENT EDITION AISC), UNLESS OTHERWISE NOTED (UON) MATERIALS SHALL BE AS FOLLOWS:	
W-SHAPES	ASTM 992, F _y =50 KSI
OTHER SHAPES & PLATES	ASTM A36, F _y =36 KSI
HSS SQUARE & RECTANGULAR SHAPES	ASTM A500 GRADE B, F _y =48 KSI
HSS ROUND SHAPES	ASTM A500 GRADE B, F _y =42 KSI
STEEL PIPES	ASTM A53 GRADE B, F _y =35 KSI
WELDING ELECTRODES	AWS A5.1 OR A5.5 SERIES E70
HIGH-STRENGTH BOLTS	3/4" ASTM A325
ANCHOR RODS	GRADE 36 ASTM F1554
WELDED STUDS	ASTM A198
DEFORMED BARS	ASTM A496
WELDABLE BARS	ASTM A706
PAINT & PROTECTION	SSPC PAINT 25
SOIL BEARING (ALLOWABLE MAXIMUM)	2,000 PSF
DEAD LOAD ONLY	2,400 PSF
DEAD LOAD PLUS LIVE LOAD	2,400 PSF
TOTAL LOAD INCLUDING WIND OR SEISMIC LOADS	3,200 PSF
SALEM ENGINEERING GROUP, REPORT DATED FEBRUARY 19, 2016	

GENERAL NOTES:	
CONCRETE:	
UNLESS OTHERWISE NOTED (UON) ON THE DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:	
FOOTINGS	3"
CANOPY SLABS	1 1/2"
SLABS ON GRADE	2" FROM TOP
ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH THE MANUAL OF STANDARD PRACTICE AND ACI 318 DURING THE PLACING OF THE CONCRETE.	
UNLESS OTHERWISE NOTED, SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS FOLLOWS:	
WELDED WIRE FABRIC	WIRE SPACING PLUS 6"
REINFORCING BARS	48 BAR DIAMETERS
ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE NOTED.	

FOUNDATION
IF FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED, UNSTABLE, OR UNSUITABLE SOIL, THE ENGINEER SHALL BE NOTIFIED.

THE BOTTOM OF ALL FOUNDATIONS SHALL EXTEND A MINIMUM OF 24 INCHES BELOW THE TOP OF FINISH GRADE.

STEPS IN WALL FOOTINGS SHALL NOT EXCEED A SLOPE OF (1) VERTICAL TO TWO (2) HORIZONTAL.

PROVIDE A MINIMUM OF TWO #4 BARS IN TOP OF CONTINUOUS WALL FOOTINGS AT DOOR AND OTHER OPENINGS, 4" LONGER THAN THE OPENING.

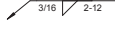
STEEL
ALL BOLTS SHALL BE SNUG TIGHT (AS DEFINED BY AISC) UON.

UNLESS OTHERWISE NOTED, BEAM-TO-BEAM AND BEAM-TO-COLUMN CONNECTIONS SHALL BE AS SHOWN IN DETAIL 6/8.0.

BEAM REACTIONS AND CONNECTIONS THAT FALL OUTSIDE OF THE LIMITS OF THE DETAILS NOTED ABOVE SHALL BE DESIGNED BY THE SPECIALTY ENGINEER LICENSED IN THE STATE OF THE PROJECT AND SHALL BE SUBMITTED AS SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL.

MINIMUM SIZE OF ALL FILLET WELDS SHALL CONFORM TO SECTION J2 AISC SPECIFICATIONS EVEN THOUGH SHOWN OTHERWISE ON ARCHITECTURAL, MECHANICAL, OR STRUCTURAL DRAWINGS.

ALL WELDS ALONG THE LENGTH OF MEMBERS INDICATED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS, BUT NOT SIZED, SHALL BE A MINIMUM OF:



STEEL STAIRS SHALL BE DESIGNED, SIGNED AND SEALED BY STEEL FABRICATORS SPECIALTY ENGINEER. SEE ARCH FOR STAIR CONFIGURATION.

PROVIDE ANGLE SUPPORT (USE L2-12x2-1/2x1/4) ALONG FACE OF STEEL COLUMNS WHERE METAL DECK OR GRATING SPANS INTO IT.

ALL STEEL JOISTS SHALL BE DESIGNED FOR THE WIND UPLIFT SHOWN. THE ROWS OF BRIDGING SHALL BE INCREASED AS REQUIRED FOR UPLIFT, SINCE A K_d FACTOR OF 0.85 HAS BEEN USED. NO INCREASE IN ALLOWABLE STRESS IS PERMITTED.

ANGLE FRAME MEMBERS AROUND TRENCHES, PITS, OPENINGS, ETC., SHALL BE MITERED, WELDED, AND GROUND SMOOTH.

END CONNECTIONS FOR WEB MEMBERS OF TRUSSES AND BRACING SHALL DEVELOP THE STRESS SHOWN OR ONE-HALF (1/2) THE ALLOWABLE STRENGTH OF THE MEMBER IN TENSION, WHICHEVER IS GREATER.

FASTENERS

EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3, SIMPSON STRONG-TIE STRONG-BOLT 2 OR APPROVED EQUAL. UON. EMBEDMENT DEPTH INTO CONCRETE OR SOLID GROUDED MASONRY SHALL BE AT LEAST 7 TIMES THE BOLT DIAMETER. CLEAN HOLE AND INSTALL PER MANUFACTURER'S PRINTED INSTRUCTIONS. ICC-ES REPORTS ESR-2002 AND ESR-1385 FOR CONCRETE AND MASONRY, RESPECTIVELY.

ADHESIVE ANCHORING (EPOXY) FOR CONCRETE SHALL BE HILTI HIT-HY 200 CARTRIDGE SYSTEM (ICC-ES REPORT ESR-1871) OR SIMPSON STRONG-TIE AT-10 (ICC-ES REPORT ESR-1082) OR APPROVED EQUAL. UON. EMBEDMENT DEPTH SHALL BE AT LEAST 12 TIMES THE INSERT DIAMETER. UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. THE HOLE SHALL BE CLEANED PER MANUFACTURER'S RECOMMENDATIONS BY BRUSHING OUT WITH WIRE BOTTLE BRUSH AND BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP (EXCEPT WHERE PERMITTED WHEN USING A HILTI SAFE SET HIT 2 ANCHOR ROD OR HILTI HOLLOW DRILL BIT W/ VACUUM). INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS AND PERFORMED BY AN INSTALLER TRAINED BY THE MANUFACTURER.

ADHESIVE ANCHORING (EPOXY) FOR MASONRY SHALL BE HILTI HIT-HY 70 CARTRIDGE SYSTEM (ICC-ES ESR-2682) OR APPROVED EQUAL. UON. EMBEDMENT DEPTH INTO SOLID GROUDED MASONRY SHALL BE AT LEAST 9 TIMES THE INSERT DIAMETER. UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. HOLES SHALL NOT BE PLACED WITHIN 1" OF A VERTICAL MORTAR JOINT. CLEAN HOLE AND INSTALL IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS AND PERFORMED BY AN INSTALLER TRAINED BY THE MANUFACTURER.

SCREW ANCHORS SHALL BE HILTI KWIK HUS-EZ, SIMPSON STRONG-TIE TITEN HD OR APPROVED EQUAL. UON. EMBEDMENT IN CONCRETE OR SOLID GROUDED MASONRY SHALL BE AT LEAST 9 TIMES THE BOLT DIAMETER. CLEAN HOLE AND INSTALL PER MANUFACTURER'S PRINTED INSTRUCTIONS.

POWER ACTUATED FASTENERS (PAF) SHALL BE 0.157" DIAMETER HILTI X-U SIMPSON STRONG-TIE POPA OR EQUAL. UON. EMBED MIN 1-1/4" INTO CONCRETE AND CMU. DO NOT PLACE WITHIN 1" OF CMU MORTAR JOINT. PAF SHALL COMPLETELY PENETRATE SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, CUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

MECHANICAL AND PLUMBING ENGINEERING, INC. OR ANY OF ITS EMPLOYEES SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

VISUAL OBSERVATIONS OF THE STRUCTURAL SYSTEM BY MCVIEGH & MANGUM ENGINEERING FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE INTERNATIONAL BUILDING CODE.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS OF EXISTING STRUCTURE AND SITES THAT ARE AFFECTED BY NEW WORK BEFORE PROCEEDING WITH FABRICATION AND CONSTRUCTION.

SPECIFICATIONS
CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION), EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE TESTING.

ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF AISC "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS", AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AND AWS D1.1 "STRUCTURAL WELDING CODE" EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS' SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.

BOLTED AND WELDED CONNECTIONS: THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO INSURE THAT ALL FASTENERS AND WELDS ARE INSTALLED IN ACCORDANCE WITH THE SPECIFICATIONS. A VISUAL INSPECTION SHALL BE MADE OF ALL WELDED AND BOLTED CONNECTIONS. IN ADDITION, A MINIMUM OF 10% OF ALL HIGH STRENGTH BOLTS SHALL BE CHECKED FOR PROPER TENSION AND A MINIMUM OF 10% OF ALL FULL PENETRATION WELDS SHALL BE CHECKED BY ULTRASONIC TESTING. ADDITIONAL BOLTS AND WELDS MAY BE TESTED AT THE OPTION OF THE TESTING LAB REPRESENTATIVE. DEFECTIVE BOLTS OR WELDS SHALL BE REMOVED AND REPLACED AT NO ADDITIONAL COST TO THE OWNER.

METAL DECK WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK. "SDI SPECIFICATIONS AND COMMENTARY FOR COMPOSITE STEEL FLOOR DECK", "SDI SPECIFICATIONS AND COMMENTARY FOR NON-COMPOSITE FLOOR DECK", AND "SDI CODE OF RECOMMENDED STANDARD PRACTICE FOR COMPOSITE DECK, FLOOR DECK AND ROOF DECK CONSTRUCTION".

FOOTING AND SLAB SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THE PROJECT PREPARED BY SALEM ENGINEERING GROUP, INC. (DATED FEBRUARY 19, 2016), AND SHALL BE IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF GOVERNING AUTHORITIES HAVING JURISDICTION.

A GEOTECHNICAL TESTING AND INSPECTION FIRM SHALL BE EMPLOYED TO PERFORM A SOIL SURVEY FOR SATISFACTORY SOIL MATERIALS, SAMPLING AND TESTING FOR QUALITY CONTROL AS PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THIS PROJECT. ALL EARTHWORK OPERATIONS SHALL BE PERFORMED TO THE SATISFACTION OF THE GEOTECHNICAL TESTING FIRM.

SHOP DRAWINGS AND SUBMITTALS
SHOP DRAWING SUBMITTALS ARE ONLY REVIEWED FOR GENERAL CONFORMANCE WITH THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR MUST REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO THEIR SUBMITTAL TO THE ARCHITECT. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP SHALL BE RETURNED WITHOUT REVIEW. ANY REQUESTED CHANGES TO THE CONTRACT DOCUMENTS SHALL BE COMMUNICATED IN WRITING PRIOR TO SUBMITTING THE SHOP DRAWINGS AND CLOUDED ON THE SHOP DRAWINGS.

SHOP DRAWINGS MUST BE SUBMITTED FOR ENGINEER'S REVIEW OF THE FOLLOWING ITEMS: (S/S = SIGNED & SEALED SHOP DRAWING WITH CALCUS, SD = SHOP DRAWING FOR REVIEW ONLY)

- CONCRETE REINFORCING LAYOUT S/S □ SD ■
- CONCRETE CONSTRUCTION JOINT LAYOUT S/S □ SD ■
- CONCRETE EMBED LAYOUT S/S □ SD ■
- CONCRETE MIX DESIGN S/S □ SD ■
- STRUCTURAL STEEL S/S □ SD ■
- STRUCTURAL STEEL CONNECTIONS S/S □ SD ■
- STEEL JOIST S/S □ SD ■
- METAL DECK S/S □ SD ■
- LIGHT GAGE METAL STUD SYSTEMS S/S □ SD □
- MISC STEEL FABRICATIONS S/S ■ SD □
- EXTERIOR CLADDING (CURTAIN WALLS) S/S ■ SD □

COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM OR RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. VENDOR SPECIFICATIONS AND COMMENTARY FOR NON-COMPOSITE FLOOR DECK, AND "SDI CODE OF RECOMMENDED STANDARD PRACTICE FOR COMPOSITE DECK, FLOOR DECK AND ROOF DECK CONSTRUCTION" SHALL BE APPROVED BY THE ENGINEER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASE STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. THE CONTRACTOR SHALL SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL.

LIGHT GAGE STEEL FRAMING

DESIGN OF LIGHT GAGE STEEL FRAMING IS BASED ON SECTION PROPERTIES AND STANDARD NOMENCLATURE AS DEFINED IN "STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) PROJECT TECHNICAL INFORMATION, ESR-3064". ALTERNATE MANUFACTURER'S FRAMING SIZE SHALL MEET THE MINIMUM SECTION PROPERTIES OF THE MEMBERS INDICATED ON THE DESIGN DRAWINGS.

THE LIGHT GAGE STEEL FRAMING SHOWN ON THE DRAWINGS IS DIAGRAMMATIC ONLY. MEMBER SIZE, CONFIGURATION, DETAILS AND CONNECTIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR'S LIGHT GAGE STEEL SPECIALTY FRAMING ENGINEER. LIGHT GAGE STEEL FRAMING SHOP DRAWINGS COMPLETE WITH CALCULATIONS, ARE TO BE SIGNED AND SEALED BY THE LIGHT GAGE STEEL SPECIALTY FRAMING ENGINEER AND SUBMITTED FOR REVIEW AND APPROVAL. ALL SUBMITTED LIGHT GAGE STEEL FRAMING SHOP DRAWINGS, WHICH ARE NOT SIGNED AND SEALED AND/OR REVIEWED BY THE GENERAL CONTRACTOR, SHALL BE REJECTED AND RETURNED TO THE GENERAL CONTRACTOR FOR RE-SUBMISSION.

ALL LIGHT GAGE STEEL FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

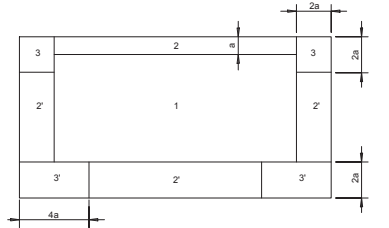
ALL FRAMING MEMBERS SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR 33 AND 43 MILL AND 50 KSI FOR 54, 68, AND 97 MILL MATERIAL.

ALL CONNECTIONS SHALL BE GALVANIZED, ASTM624 WELDED (MIN).

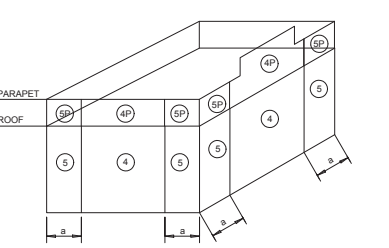
ALL CONNECTIONS SHALL BE SCREWED OR COATED. USE MINIMUM OF #410 SCREWS AT EA CONNECTION. POWER DRIVEN FASTENERS (PDF) SHALL COMPLETELY PENETRATE THE STRUCTURAL STEEL.

ALL CONNECTIONS NOT SHOWN HEREIN, OR ANY DESIRED SUBSTITUTIONS SHALL BE ENGINEERED, DETAILED, SUBMITTED, AND SIGNED AND SEALED BY A REGISTERED ENGINEER IN THE STATE OF THE PROJECT.

ALL WALLS SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT.
WELLS COMPOSED OF LIGHT GAGE STEEL STUDS SHALL HAVE LATERAL BRACING INSTALLED AT A MAXIMUM SPACING OF 48".



ROOF PLAN (GENERIC BUILDING SHOWN)



WALLS (GENERIC BUILDING SHOWN)



COMPONENTS AND CLADDING LOADS
SCALE: 3/4" = 1'-0"

COMPONENT & CLADDING DESIGN WIND PRESSURES (PSF)

ROOF			
ZONE	10 SF	50 SF	100 SF
1	16.0 psf	16.0 psf	16.0 psf
	-31.7 psf	-31.7 psf	-31.7 psf
2	16.0 psf	16.0 psf	16.0 psf
	-38.7 psf	-35.0 psf	-34.2 psf
3	16.0 psf	16.0 psf	16.0 psf
	-49.1 psf	-38.0 psf	-34.2 psf
NEGATIVE ZONE 2'	-44.1 psf	-42.4 psf	-41.7 psf
NEGATIVE ZONE 3'	-68.9 psf	-51.6 psf	-44.1 psf

WALLS			
ZONE	10 SF	100 SF	500 SF
4	26.8 psf	22.8 psf	20.1 psf
	-29.0 psf	-25.0 psf	-22.3 psf
4p	57.2 psf	50.4 psf	47.6 psf
	-49.1 psf	-40.8 psf	-35.1 psf
5	26.8 psf	22.8 psf	20.1 psf
	-35.7 psf	-27.8 psf	-22.3 psf
5p	70.2 psf	50.4 psf	47.6 psf
	-56.2 psf	-43.7 psf	-35.1 psf

- NOTES:
- 1) TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY AREAS, LINEARLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE.
 - 2) POSITIVE PRESSURES ACT TOWARD THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
 - 3) SEE DIAGRAMS FOR LOCATION OF ZONES.
 - 4) PRESSURES SHOWN ARE ULTIMATE PRESSURES. MULTIPLY BY 0.6 FOR NOMINAL PRESSURES.
- a=6'-3"

STRUCTURAL ABBREVIATIONS

#	NUMBER OR ROUND	GA	GAGE	SCHED	SCHEDULE
Ø	ROUND OR DIAMETER	GALV	GALVANIZED	SECT	SECTION
□	SQUARE	CC	GENERAL CONTRACTOR	SIM	SIMILAR
△	TRIANGLE	GT	GENERAL TRUSS	SPE	SPECIFICATION
⊙	AT	HGT	HEIGHT	SQL	SQUARE
ALUM	ALUMINUM	HORIZ	HORIZONTAL	STD	STANDARD
AR	ANCHOR ROD	HSA	HEADED STUD ANCHOR	STIFF	STIFFENER
ARCH	ARCHITECTURE	HT	HEIGHT	STR	STRAIGHT
ASSY	ASSEMBLY	IF	INFO	STRUCT	STRUCTURAL
B/	BOTTOM OF	INT	INTERIOR	SYM	SYMMETRICAL
BETWN	BETWEEN	INT	INTERIOR	TAB	TOP & BOTTOM
BLDG	BUILDING	JNT	JOINT	TOP	TOP OF
BM	BEAM	JST	JOIST	TU	TILT-UP PANEL
BOT	BOTTOM	LB	LONG	TE	THICKENED EDGE
BPL	BASE PLATE	LG	LONG LEG	THK	THICK
BRG	BEARING	LLH	LONG LEG HORIZONTAL	THRD	THREADED
C	CANTO	LLV	LONG LEG VERTICAL	TRANSV	TRANSVERSE
C TO C	CENTER TO CENTER	LSH	LONG SIDE HORIZONTAL	TS	THICKENED SLAB
CANTILEVER	CANTILEVER	LSV	LONG SIDE VERTICAL	TYP	TYPICAL
CONSTRUCTION JOINT	CONSTRUCTION JOINT	MANUF	MANUFACTURER	UON	UNLESS OTHERWISE NOTED
CLR	CLEAR	MAT'L	MATERIAL	VER	VERTICAL
CMU	CONCRETE MASONRY UNIT	MAX	MAXIMUM	W/	WITH
COL	COLUMN	MECH	MECHANICAL	W/O	WITHOUT
CONC	CONCRETE	MIN	MINIMUM	WP	WORK POINT
CONN	CONNECTION	MISC	MISCELLANEOUS	WS	WATER STOP
CONSTR	CONSTRUCTION	NIC	NOT IN CONTRACT	WWF	WELDED WIRE FABRIC
CONT	CONTINUOUS	NO	NO		
CONTR	CONTRACTOR	NTS	NOT TO SCALE		
CTR	CENTER	OC	ON CENTER		
CTRD	CENTERED	OD	ON DIAMETER		
DBA	DEFORMED BAR ANCHOR	OF	OUTSIDE FACE		
DEFL	DEFLECTION	O/Q	OPENING		
DETA	DETAIL	OPP	OPPOSITE		
DIA	DIAMETER	PERP	PERPENDICULAR		
DIAG	DIAGONAL	PL	PANEL		
DIFF	DIFFERENT	PREF	PREFABRICATED		
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT		
DD	DITTO	PSI	POUNDS PER SQUARE INCH		
DWG	DRAWING	QTY	QUANTITY		
EA	EACH	R	RADIUS		
EACH FACE	EACH FACE	RD	ROUND		
EXPANSION JOINT	EXPANSION JOINT	RO	ROUND OVER		
ELEV	ELEVATION	REF	REFERENCE		

SCHEDULE OF SPECIAL INSPECTION SERVICES					
PROJECT	SERVICE	APPLICABLE TO THIS PROJECT			
		Y/N	EXTENT	AGENT*	DATE COMPLETED
1704.2.5 Inspection of Fabricators					
Verify fabrication/quality control procedures	In-plant review (3)	Y			
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)	Submit review, shop (3) and/or field inspection	Y			
1705.2 Steel Construction					
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submit Review	Y	Each submittal		
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic		
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Periodic		
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Y	Periodic		
5. Structural steel welding:					
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)		
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Y	Observe (4)		
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)		
d. Nondestructive testing (NDT) of welded joints: see Commentary					
1) Complete penetration groove welds 5/16" or greater in risk category III or IV	Shop (3) or field ultrasonic testing - 100%		Periodic		
2) Complete penetration groove welds 5/16" or greater in risk category II	Shop (3) or field ultrasonic testing - 10% of welds minimum		Periodic		
3) Thermally cut surfaces of access holes when material t > 2"	Shop (3) or field magnetic Partical or Penetrant testing		Periodic		
4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing		Periodic		
5) Fabricator's NDT reports when fabricator performs NDT	Verify reports		Each submittal (5)		
6. Structural steel bolting:	Shop (3) and field inspection				
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Y	Observe or Perform as noted (4)		
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Y	Observe (4)		
1) Pre-tensioned and slip-critical joints					
a) Turn-of-nut with matching markings			Periodic		
b) Direct tension indicator			Periodic		
c) Twist-off type tension control bolt			Periodic		
d) Turn-of-nut without matching markings			Continuous		
e) Calibrated wrench			Continuous		
2) Snug-tight joints		Y	Periodic		
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Y	Perform (4)		
7. Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Shop (3) and field inspection and testing		Observe or Perform as noted (4)		
1705.2.2 Steel Construction Other Than Structural Steel					
1. Material verification of cold-formed steel deck:					
a. Identification markings	Field inspection		Periodic		
b. Manufacturer's certified test reports	Submit Review		Each submittal		
2. Connection of cold-formed steel deck to supporting structure:	Shop (3) and field inspection		Periodic		
a. Welding					
b. Other fasteners (in accordance with AISC 360, Section N6)					
1) Verify fasteners are in conformance with approved submittal			Periodic		
2) Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations			Periodic		
3. Reinforcing steel:	Shop (3) and field inspection				
a. Verification of weldability of steel other than ASTM A706			Periodic		
b. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement			Continuous		
c. Shear reinforcement			Continuous		
d. Other reinforcing steel			Periodic		

4. Cold-formed steel trusses spanning 60 feet or greater					
a. Verify temporary and permanent restraints/bracing are installed in accordance with the approved truss submittal package	Field inspection		Periodic		
1705.3 Concrete Construction					
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Y	Periodic		
2. Inspection of prestressing steel installation	Shop (3) and field inspection		Periodic		
3. Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop (3) and field inspection	Y	Periodic		
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Y	Periodic or as required by the research report issued by an approved source		
5. Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic		
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Y	Continuous		
7. Inspection of concrete and shcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous		
8. Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Y	Periodic		
9. Inspection of prestressed concrete:	Shop (3) and field inspection				
a. Application of prestressing force			Continuous		
b. Grouting of bonded prestressing tendons in the seismic-force-resisting system			Continuous		
10. Erection of precast concrete members	Field inspection		"In accordance" with construction documents		
a. Inspect in accordance with construction documents	Field inspection		"In accordance" with Section 1705.2		
b. Perform inspections of welding and bolting in accordance with Section 1705.2	Field inspection		"In accordance" with Section 1705.2		
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports		Periodic		
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic		
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic		
1705.4 Masonry Construction					
(A) Level A, B and C Quality Assurance:					
1. Verify compliance with approved submittals	Field inspection		Periodic		
(B) Level B Quality Assurance:					
1. Verification of fm and Fc prior to construction	Testing by unit strength method or prism test method		Periodic		
(C) Level C Quality Assurance:					
1. Verification of fm and Fc prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method		Periodic		
2. Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self-consolidating grout, as delivered to the project site	Field inspection		Continuous		
3. Verify placement of masonry units	Field inspection		Periodic		
(D) Levels B and C Quality Assurance:					
1. Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered to the project	Field testing		Continuous		
2. Verify compliance with approved submittals	Field inspection		Periodic		
3. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field inspection		Periodic		
4. Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Field inspection		Periodic		
5. Verify construction of mortar joints	Field inspection		Periodic		
6. Verify placement of reinforcement, connections, and prestressing tendons and anchorages	Field inspection		Level B - Periodic Level C - Continuous		
7. Verify grout space prior to grouting	Field inspection		Level B - Periodic Level C - Continuous		
8. Verify placement of grout and prestressing grout for bonded tendons	Field inspection		Continuous		
9. Verify size and location of structural masonry elements					

9. Verify size and location of structural masonry elements	Field Inspection		Periodic		
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field Inspection		Level B - Periodic Level C - Continuous		
11. Verify welding of reinforcement (see 1705.2.2)	Field Inspection		Continuous		
12. Verify preparation, construction, and protection of masonry during cold weather (temperature below 45F) or hot weather (temperature above 95F)	Field inspection		Periodic		
13. Verify application and measurement of prestressing force	Field Inspection		Continuous		
14. Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry)	Field inspection		Continuous		
15. Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first 5000 SF of AAC masonry)	Field inspection		Level B - Periodic Level C - Continuous		
16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Field inspection		Continuous		
17. Verify properties of thin-bed mortar for AAC masonry (after the first 5000 SF of AAC masonry)	Field inspection		Level B - Periodic Level C - Continuous		
18. Prepare grout and mortar specimens	Field testing		Level B - Periodic Level C - Continuous		
19. Observe preparation of prisms	Field inspection		Level B - Periodic Level C - Continuous		
1705.5 Wood Construction					
1. Inspection of the fabrication process of wood structural elements and assemblies in accordance with Section 1704.2.5	In-plant review (3)		Periodic		
2. For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans	Field inspection		Periodic		
3. For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection		Periodic		
4. Metal-plate-connected wood trusses spanning 60 feet or greater: Verify temporary and permanent restraints/bracing are installed in accordance with the approved truss submittal package	Field inspection		Periodic		
1705.6 Soils					
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Y	Periodic		
2. Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Y	Periodic		
3. Perform classification and testing of controlled fill materials.	Field inspection	Y	Periodic		
4. Verify use of proper materials, densities, and fit thicknesses during placement and compaction of controlled fill	Field inspection	Y	Continuous		
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic		
1705.7 Driven Deep Foundations					
1. Verify element materials, sizes and lengths comply with requirements	Field inspection		Continuous		
2. Determine capacities of test elements and conduct additional load tests, as required	Field inspection		Continuous		
3. Observe driving operations and maintain complete and accurate records for each element	Field inspection		Continuous		
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element	Field inspection		Continuous		
5. For steel elements, perform additional inspections per Section 1705.2	See Section 1705.2		See Section 1705.2		
6. For concrete elements and concrete-filled elements, perform additional inspections per Section 1705.3	See Section 1705.3		See Section 1705.3		
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge	Field inspection		In accordance with construction documents		
8. Perform additional inspections and tests in accordance with the construction documents	Field inspection and testing		In accordance with construction documents		

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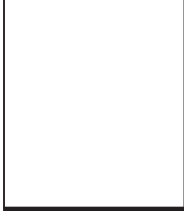


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Revisions:

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Principal in Charge:

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Project Manager:

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Sheet Number:

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1705.8 Cast-in-Place Deep Foundations 1. Observe drilling operations and maintain complete and accurate records for each element	Field Inspection	Continuous			
2. Verify placement locations and plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	Field Inspection	Continuous			
3. For concrete elements, perform additional inspections in accordance with Section 1705.3	See Section 1705.3	See Section 1705.3			
4. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	In accordance with construction documents			
1705.9 Helical Pile Foundations 1. Verify installation equipment, pile dimensions, tip elevations, final depth, final installation torque and other data as required.	Field Inspection	Continuous			
2. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	In accordance with construction documents			
1705.10.1 Structural Wood Special Inspections For Wind Resistance 1. Inspection of field gluing operations of elements of the main windforce-resisting system	Field Inspection	Continuous			
2. Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Periodic			
1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance 1. Inspection during welding operations of elements of the main windforce-resisting system	Shop (3) and field inspection	Periodic			
2. Inspections for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Periodic			
1705.10.3 Wind-resisting Components 1. Roof cladding	Shop (3) and field inspection	Periodic			
2. Wall cladding	Shop (3) and field inspection	Periodic			
1705.11.1 Structural Steel Special Inspections for Seismic Resistance Inspection of structural steel in accordance with AISC 341	Shop (3) and field inspection	In accordance with AISC 341			
1705.11.2 Structural Wood Special Inspections for Seismic Resistance 1. Inspection of field gluing operations of elements of the seismic-force resisting system	Field Inspection	Continuous			
2. Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	Periodic			
1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance 1. Inspection during welding operations of elements of the seismic-force-resisting system	Shop (3) and field inspection	Periodic			
2. Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	Periodic			
1705.11.4 Designated Seismic Systems Verification Inspect and verify that that the component label, anchorage or mounting conforms to the certificate of compliance in accordance with Section 1705.12.3	Field Inspection	Periodic			
1705.11.5 Architectural Components Special Inspections for Seismic Resistance 1. Inspection during the erection and fastening of exterior cladding and interior and exterior veneer	Field Inspection	Periodic			
2. Inspection during the erection and fastening of interior and exterior nonbearing walls	Field Inspection	Periodic			
3. Inspection during anchorage of access floors	Field Inspection	Periodic			


1705.11.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance 1. Inspection during the anchorage of electrical equipment for emergency or standby power systems	Field Inspection	Periodic			
2. Inspection during the anchorage of other electrical equipment	Field Inspection	Periodic			
3. Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units	Field Inspection	Periodic			
4. Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials	Field Inspection	Periodic			
5. Inspection during the installation and anchorage of vibration isolation systems	Field Inspection	Periodic			
1705.11.7 Storage Racks Special Inspections for Seismic Resistance Inspection during the anchorage of storage racks 8 feet or greater in height	Field Inspection	Periodic			
1705.11.8 Seismic Isolation Systems Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system	Shop and field inspection	Periodic			
1705.12.1 Concrete Reinforcement Testing and Qualification for Seismic Resistance 1. Review certified mill test reports for each shipment of reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review certified mill test reports	Each shipment			
2. Verify reinforcement weldability of ASTM A615 reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review test reports	Each shipment			
1705.12.2 Structural Steel Testing and Qualification for Seismic Resistance Test in accordance with the quality assurance requirements of AISC 341	Shop (3) and field testing	Per AISC 341			
1705.12.3 Seismic Certification of Nonstructural Components Review certificate of compliance for designated seismic system components.	Certificate of compliance review	Each submittal			
1705.12.4 Seismic Isolation Systems Test seismic isolation system in accordance with ASCE 7 Section 17.8	Prototype testing	Per ASCE 7			
1705.13 Sprayed Fire-resistant Materials 1. Verify surface condition preparation of structural members	Field Inspection	Periodic			
2. Verify application of sprayed fire-resistant materials	Field Inspection	Periodic			
3. Verify average thickness of sprayed fire-resistant materials applied to structural members	Field Inspection	Periodic			
4. Verify density of the sprayed fire-resistant material complies with approved fire-resistant design	Field Inspection and testing	Per IBC Section 1705.13.5			
5. Verify the cohesive/adhesive bond strength of the cured sprayed fire-resistant material	Field Inspection and testing	Per IBC Section 1705.13.6			
1705.14 Mastic and Intumescent Fire-Resistant Coatings Inspect mastic and intumescent fire-resistant coatings applied to structural elements and decks	Field Inspection	Periodic			
1705.15 Exterior Insulation and Finish Systems (EIFS) 1. Verify materials, details and installations are per the approved construction documents	Field Inspection	Periodic			
2. Inspection of water-resistive barrier over sheathing substrate	Field Inspection	Periodic			
1705.16 Fire-Resistant Penetrations and Joints 1. Inspect penetration firestop systems	Field testing	Per ASTM E2174			
2. Inspect fire-resistant joint systems	Field testing	Per ASTM E2393			
1705.17 Smoke Control Systems 1. Leakage testing and recording of device locations prior to concealment	Field testing	Periodic			
2. Prior to occupancy and after sufficient completion, pressure difference testing, flow measurements, and detection and control verification	Field testing	Periodic			

INSPECTION AGENTS	FIRM	ADDRESS	TELEPHONE NO.
1.			
2.			
3.			
4.			

Notes: 1. The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.
2. The list of Special Inspectors may be submitted as a separate document, if noted so above.
3. Special Inspectors as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2
4. Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.
5. NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.

Are Requirements for Seismic Resistance included in the Statement of Special Inspections? **No**
Are Requirements for Wind Resistance included in the Statement of Special Inspections? **No**
DATE: OCT 26, 2015

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PROJECT MANAGER:
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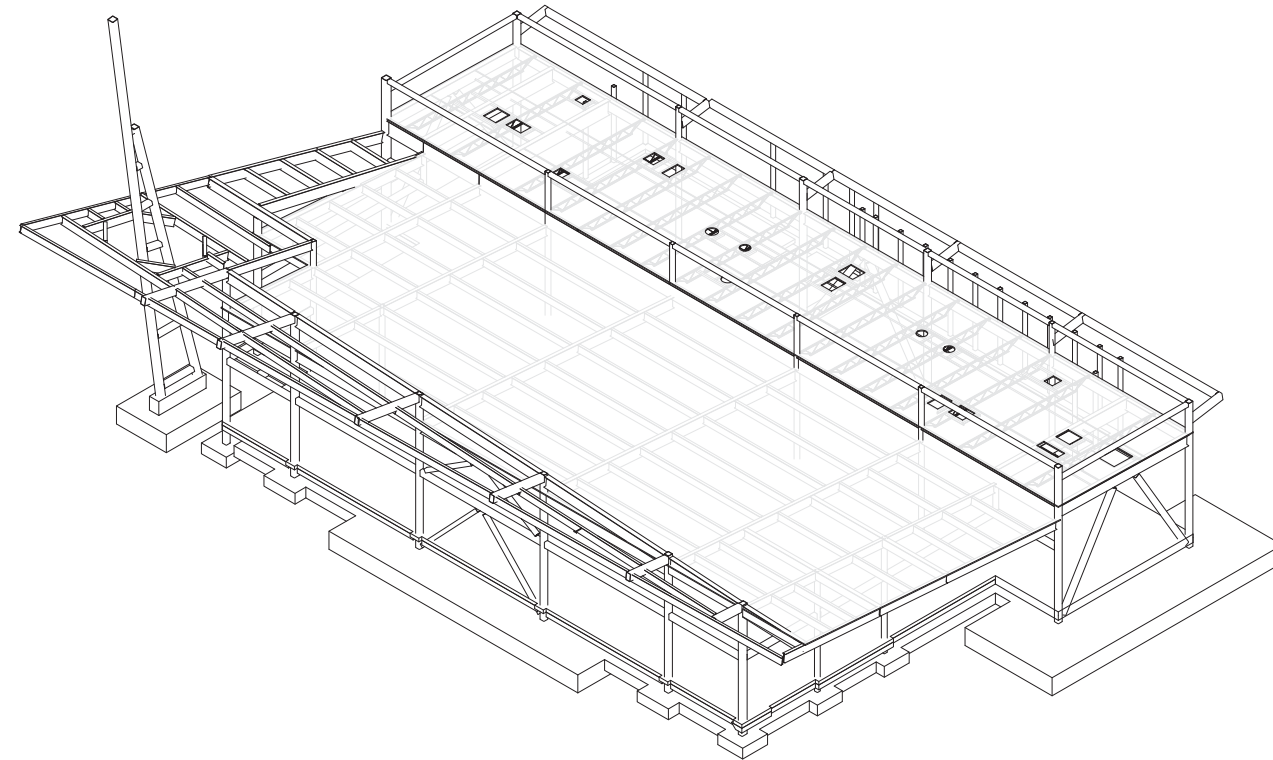
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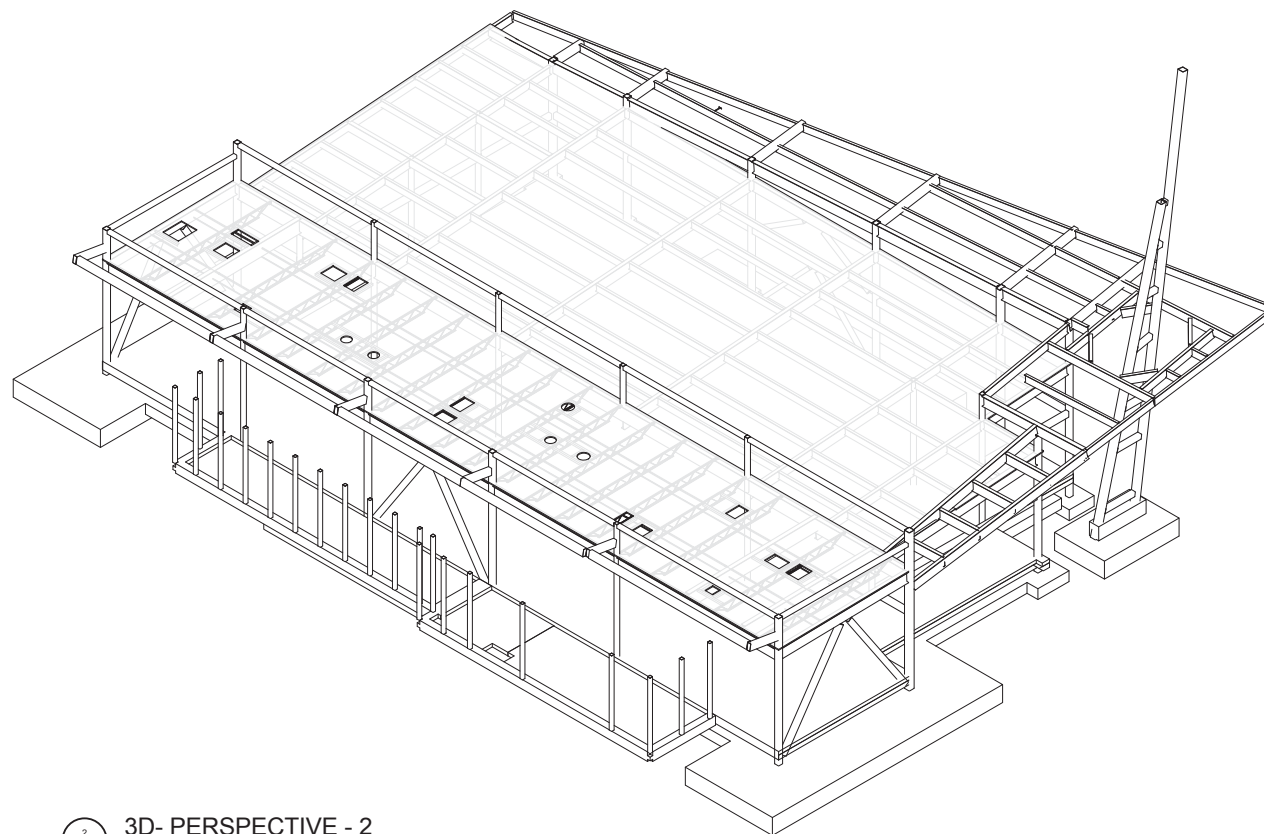
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1
S0.4
3D- PERSPECTIVE - 1
SCALE:



2
S0.4
3D- PERSPECTIVE - 2
SCALE:



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DESIGNER:

WESLEY STEPHENS

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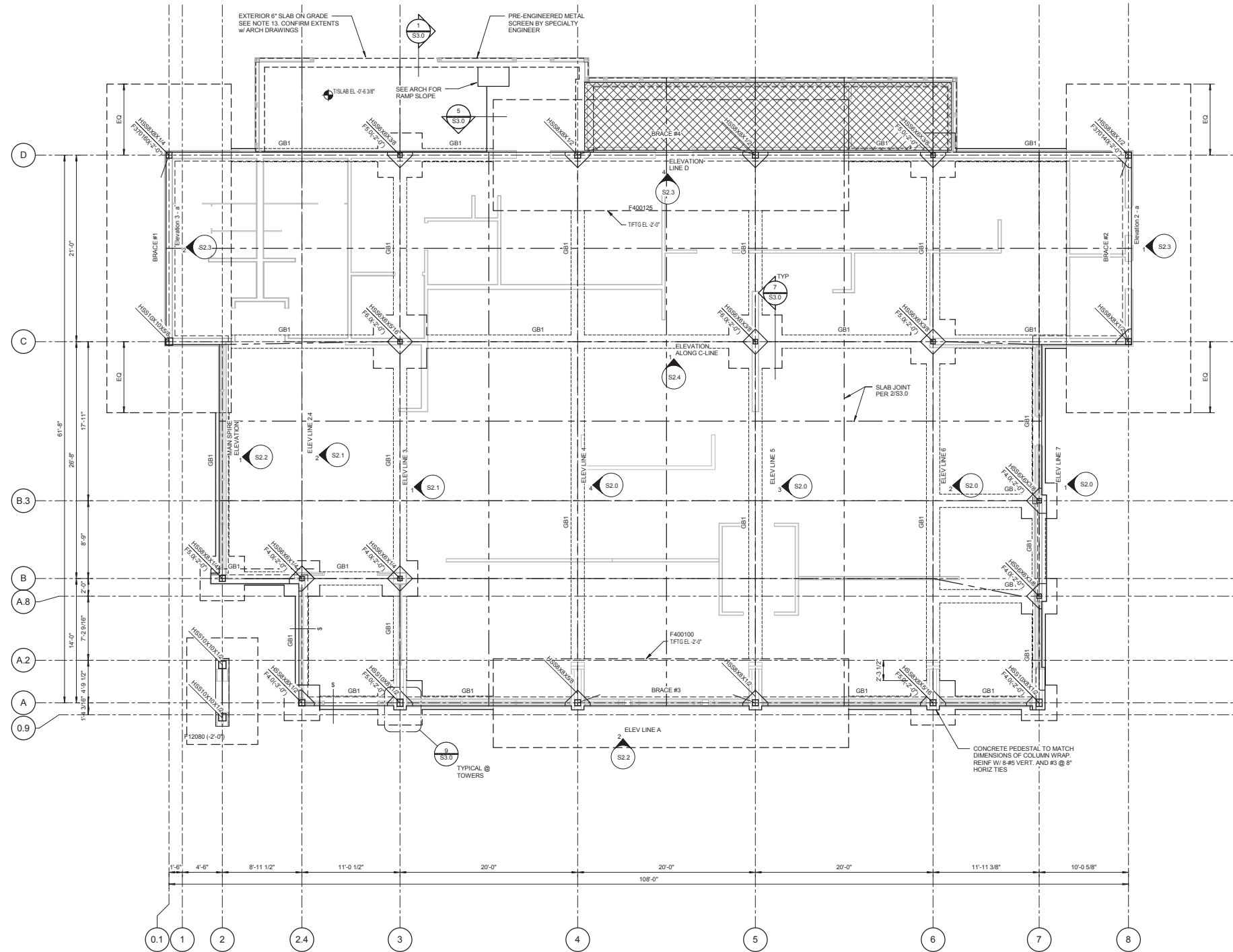
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PERSPECTIVE VIEWS

Sheet Number:

S0.4



1 FOUNDATION PLAN
SCALE: 3/16" = 1'-0"

GRADE BEAM SCHEDULE				
Type	BEAM DEPTH	BEAM WIDTH	REINFORCING	TIES
(Thickened Concrete Slab) 12x16	1'-4"	1'-0"		
GB1	1'-6"	1'-6"	8-#6's	#3's @ 18" o.c.

FOOTING SCHEDULE								
TYPE	LENGTH	WIDTH	THICKNESS	BOTTOM REINF (LONG WAY)	BOTTOM REINF (SHORT WAY)	TOP REINF (LONG WAY)	TOP REINF (SHORT WAY)	REMARKS
F4.0	4'-0"	4'-0"	1'-6"	12-#5	12-#5			
F5.0	5'-0"	5'-0"	1'-6"	15-#5	15-#5			
F6.0	6'-0"	6'-0"	1'-6"	8-#5	8-#5			
F12080	12'-0"	8'-0"	2'-6"	8-#5 @ 12" o.c.	8-#5 @ 12" o.c.	8-#5 @ 12" o.c.	8-#5 @ 12" o.c.	
F370140	37'-0"	14'-0"	2'-6"	14-#7	#7's @ 12" o.c.	15-#5	#5's @ 12" o.c.	
F370160	37'-0"	16'-0"	2'-6"	19-#5	#7's @ 12" o.c.	17-#5	#5's @ 12" o.c.	
F400100	40'-0"	10'-0"	2'-6"	17-#8	#8's @ 12" o.c.	10-#6	#5's @ 12" o.c.	
F400125	40'-0"	12'-6"	2'-6"	14-#8	#8's @ 12" o.c.	10-#6	#5's @ 12" o.c.	

FOUNDATION PLAN NOTES:

- ELEVATIONS ARE BASED ON TISLAB DATUM EL 0'-0". SEE CIVIL DWGS.
- FF & W/F DENOTES FOOTING TYPES. SEE SCHEDULE THIS SHEET. TOP OF FOOTING EL = -2'-0" TYP. UON.
- FOR TYPICAL COLUMN BASE PLATES, SEE SHEET S3.0.
- FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEET S0.1.
- COLUMNS SIZES ARE NOTED ON PLAN.
- FOR STEEL BRACE FRAMES AND MOMENT FRAMES, SEE SHEETS S2.1, S2.2, S2.3 & S2.4.
- GBF DENOTES GRADE BEAM TYPES. SEE SCHEDULE THIS SHEET. TOP OF GRADE BEAM EL = -2'-0" TYP. UON.
- SEE SHEET 10/S3.0 FOR REINF @ GRADE BEAM AND FOOTING CORNERS.
- PROVIDE #4@2'-0" MID SLAB @ ALL RE-ENTRANT CORNERS.
- SEE SHEET S3.0 FOR PIPE PENETRATIONS THROUGH BUILDING PERIMETER.
- FOR DIMENSIONS, SECTIONS & ELEVATIONS NOT SHOWN, SEE ARCH.
- CONC. SLAB ON SHALL BE 4" NW CONCRETE W/ #3 @ 18" o.c. E.W. FOR INTERIOR SLABS. EXTERIOR SLAB SHALL BE 6" NW CONCRETE WITH #4's @ 18" o.c. SLABS SHALL BE SET ON A 4" COMPACTED GRANULAR BASE IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT.
- § DENOTES STEP IN GRADE BEAM. SEE DETAIL 4/S3.0.



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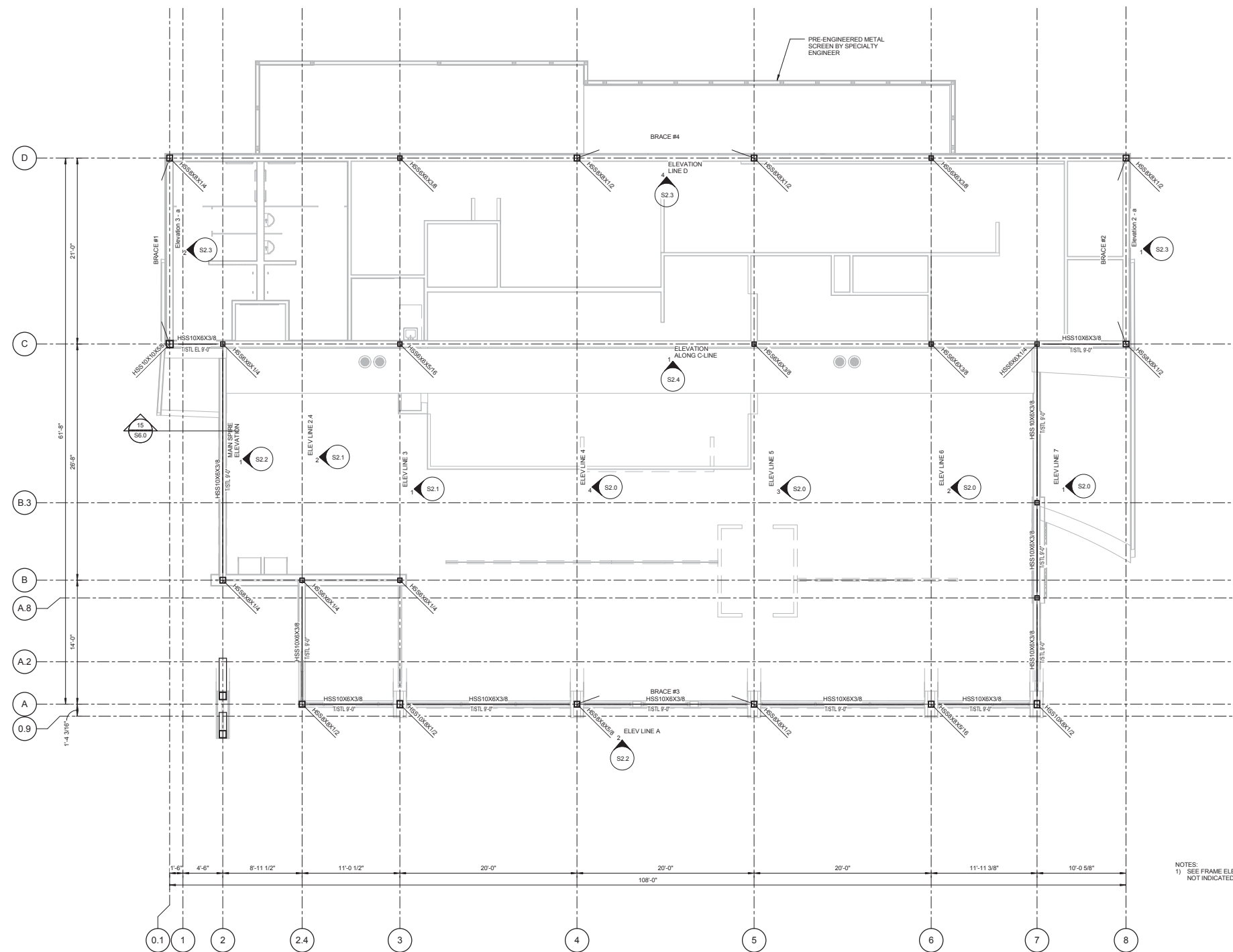
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GROUND FLOOR &
FOUNDATION PLAN

Sheet Number:
S1.1



1 INTERMEDIATE FRAMING
SCALE: 3/16" = 1'-0"

NOTES:
1) SEE FRAME ELEVATIONS FOR INFORMATION NOT INDICATED

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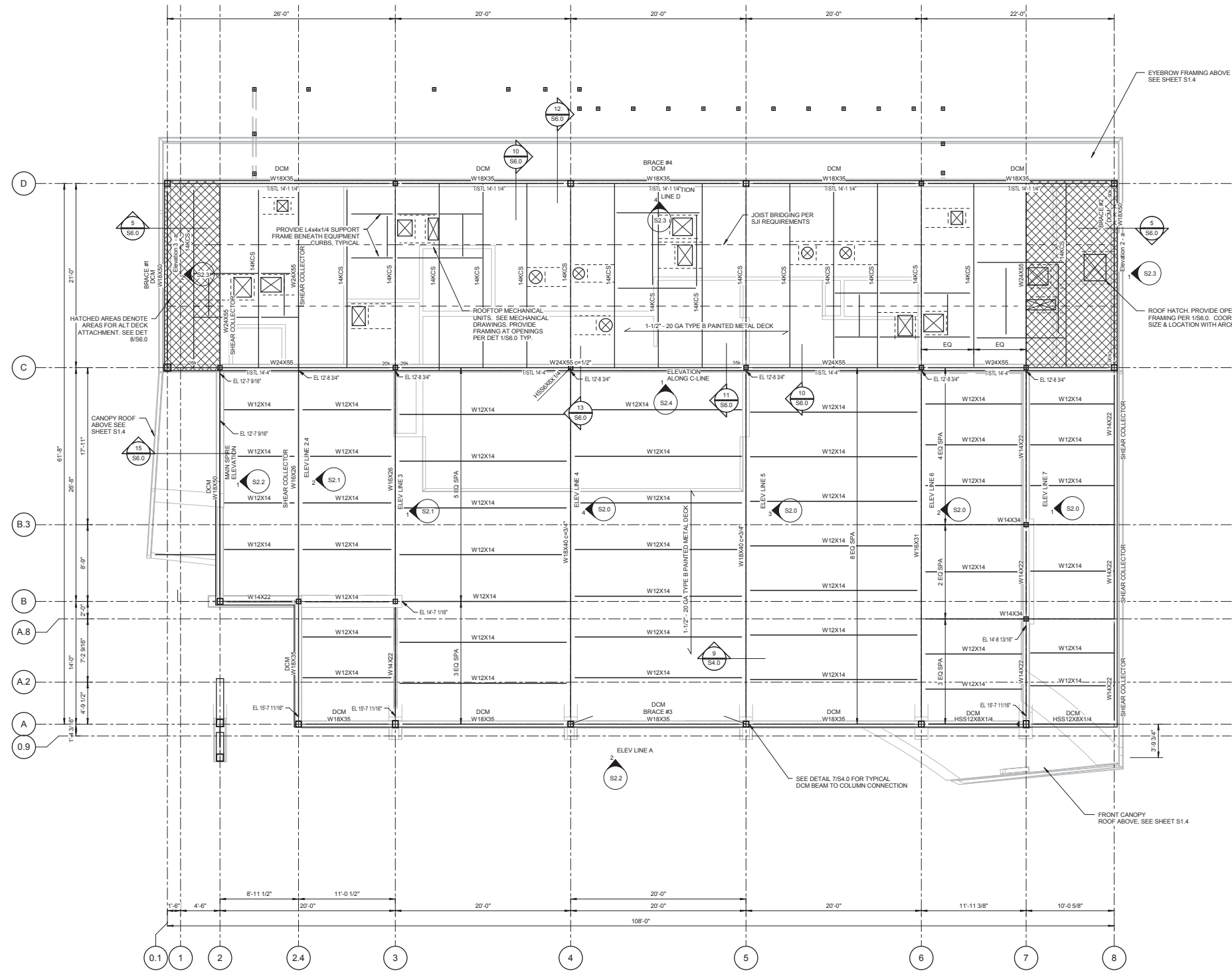
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 INTERMEDIATE
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Sheet Number:
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1 ROOF FRAMING PLAN
SCALE: 3/16" = 1'-0"

- ROOF FRAMING PLAN NOTES:
- ELEVATIONS ARE BASED ON TOP OF SLAB ON GRADE DATUM 0'-0" AT T/S/LB LEVEL 1.
 - TOP OF STEEL (T/S/L) ELEVATION VARIES LINEARLY; SEE PLAN.
 - FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEET S0.1.
 - FOR FRAMING AT ROOF PENETRATIONS NOT SHOWN, SEE 1/8" & 2/8" COORDINATE AND VERIFY OPENINGS WITH ARCH AND MECH DIVS.
 - ROOF DECK ATTACHMENT SHALL BE PER DECK ATTACHMENT SCHEDULE SEE DETAIL S15.0.
 - WHERE BRACING IS INTERRUPTED BY MECHANICAL UNITS, ROOF OPENINGS OR OTHERWISE, PROVIDE SPECIAL BRIDGING DESIGN TO ACCOMMODATE SPECIAL CONDITIONS. SUCH SPECIAL BRIDGING SHALL BE SHOWN ON SHOP DRAWINGS SUBMITTED FOR ENGINEER REVIEW AND APPROVAL.
 - VERIFY ALL DIMENSIONS AFFECTED BY MECHANICAL WITH ACTUAL EQUIPMENT PURCHASED PRIOR TO FABRICATION OF STEEL FRAMING.
 - BEAMS AND OTHER FRAMING MEMBERS WERE DESIGNED TO ACCOMMODATE A MISCELLANEOUS HANGING LOAD (INCLUDING DUCTWORK, SPRINKLER SYSTEM, PLUMBING, CEILING & OTHER MISCELLANEOUS HANGING COMPONENTS) OF 8 PSF. HANGING LOADS SHALL BE DISTRIBUTED AS TO NOT EXCEED THIS VALUE.
 - FOR TYPICAL BEAM TO BEAM & BEAM TO COLUMN CONNECTION, SEE DETAIL S16.0.
 - SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, SECTIONS, AND ELEVATIONS NOT SHOWN.
 - METAL DECK SHALL BE 20 GA. 1 1/2" B-TYPE DECK, LINO.
 - "DCM" DENOTES DIAPHRAGM CHORD MEMBER. DCM'S ARE ALSO SHEAR COLLECTORS, UNO. SHEAR COLLECTORS ONLY ARE NOTED ACCORDINGLY.
 - DENOTES FACTORED BEAM REACTION FOR SIZING BEAM CONNECTION. REACTIONS NOT LISTED SHALL BE DESIGN FOR A FACTORED BEAM REACTION OF 20K.
 - DENOTES MOMENT CONNECTION, SEE 4/S4.0 UCN.



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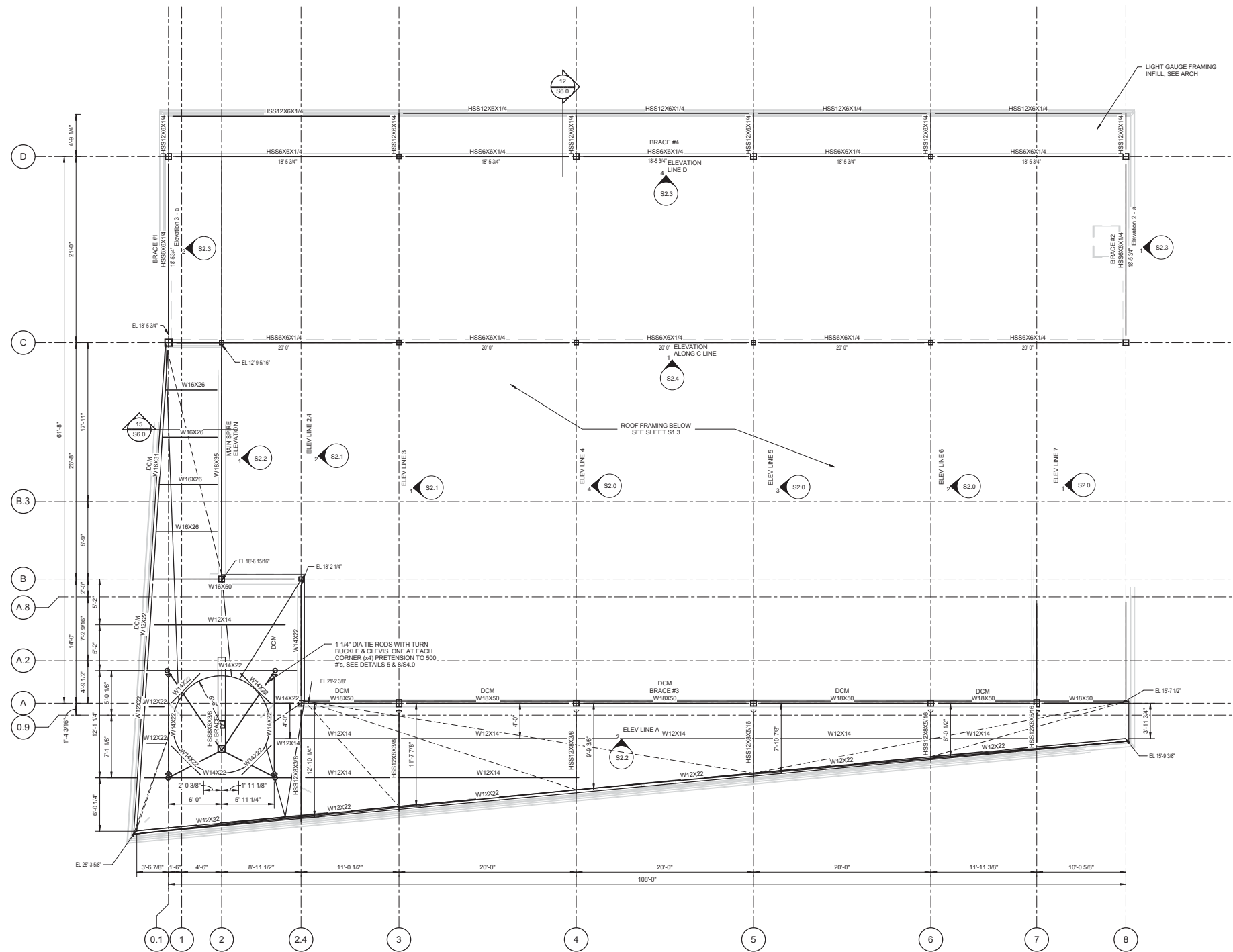
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Sheet Title:
ROOF FRAMING PLAN

Sheet Number:
S1.3



1
S1.4 KITCHEN PARAPET & FRONT CANOPY FRMNG PLAN
SCALE: 3/16" = 1'-0"

NOTE: SEE SHEET S1.3 FOR PLAN NOTES.

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Designer:

WESLEY STEPHENS

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Sheet Title:

KITCHEN PARAPET

PLAN & FRONT

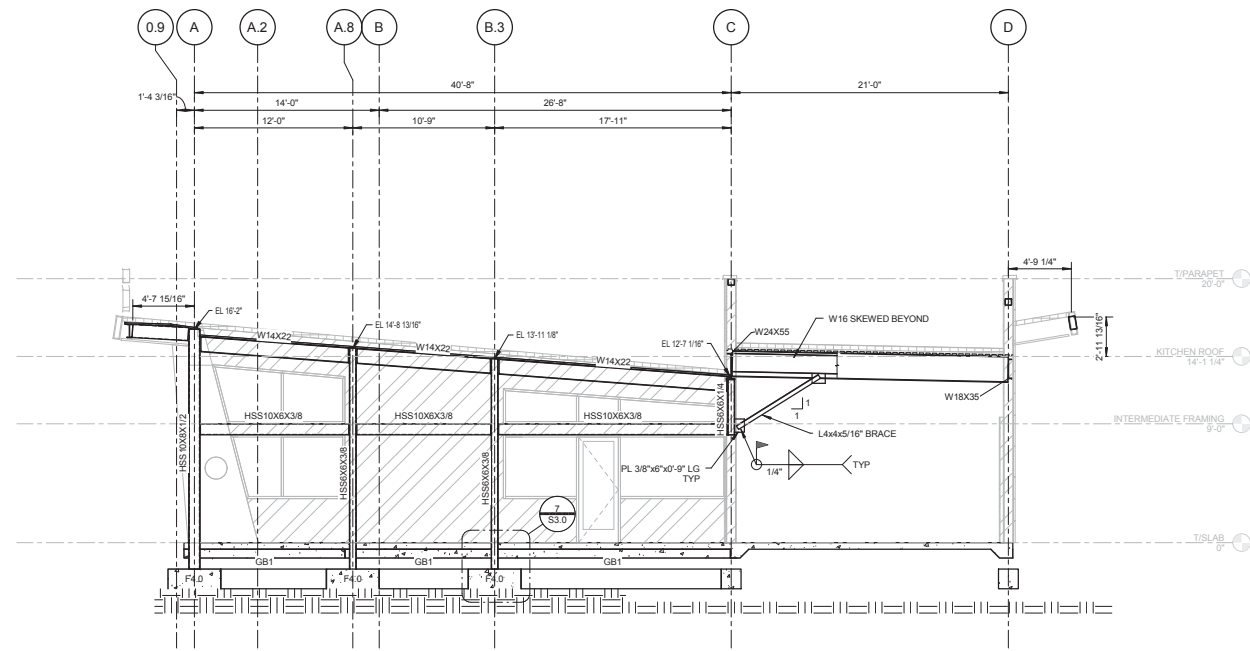
CANOPY FRAMING

PLAN

Sheet Number:

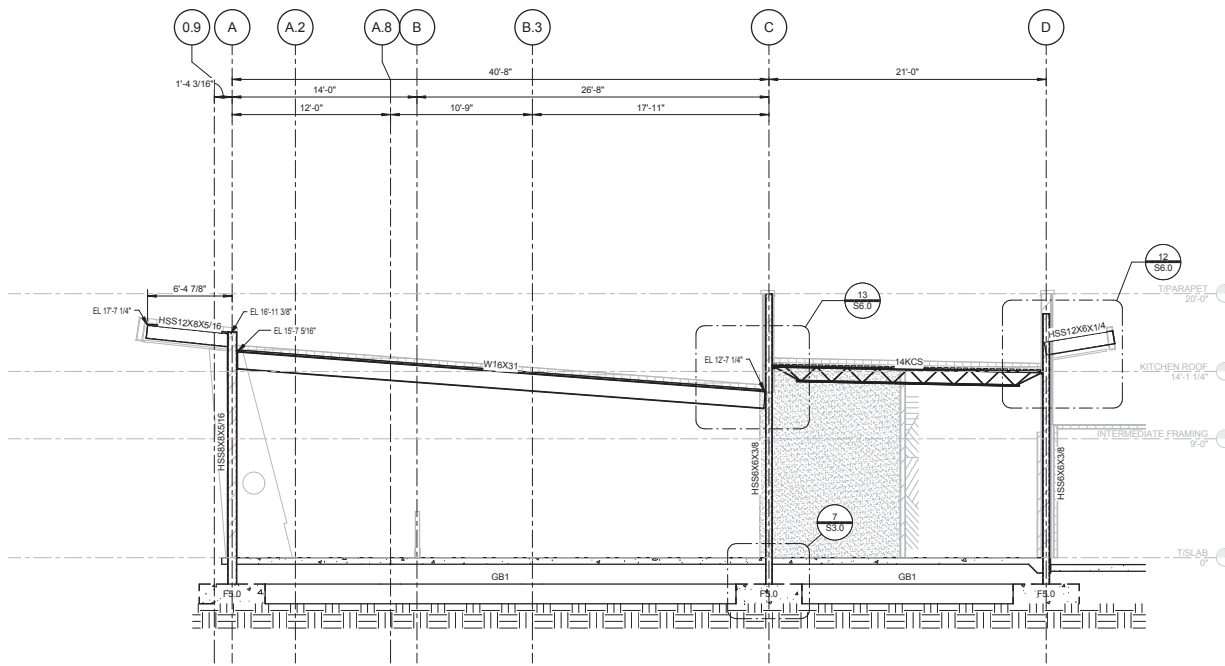
S1.4





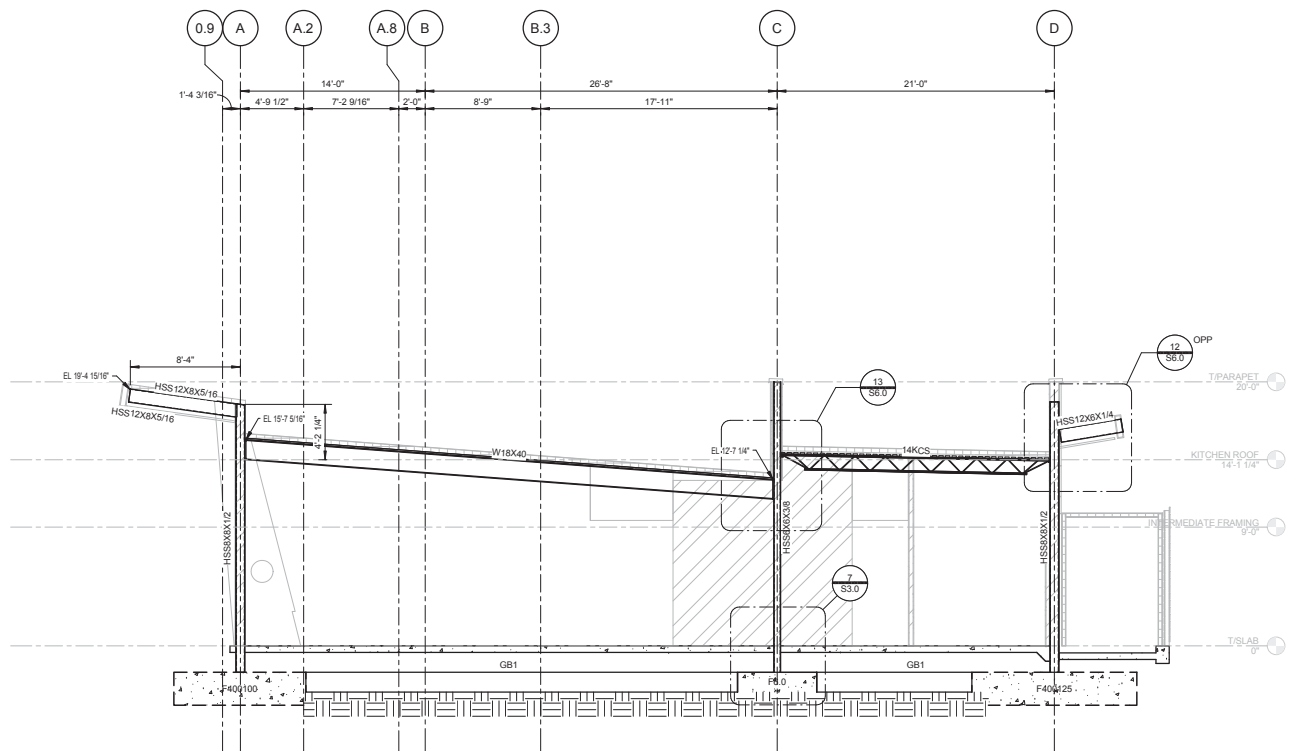
NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

1 ELEV LINE 7
SCALE: 3/16" = 1'-0"



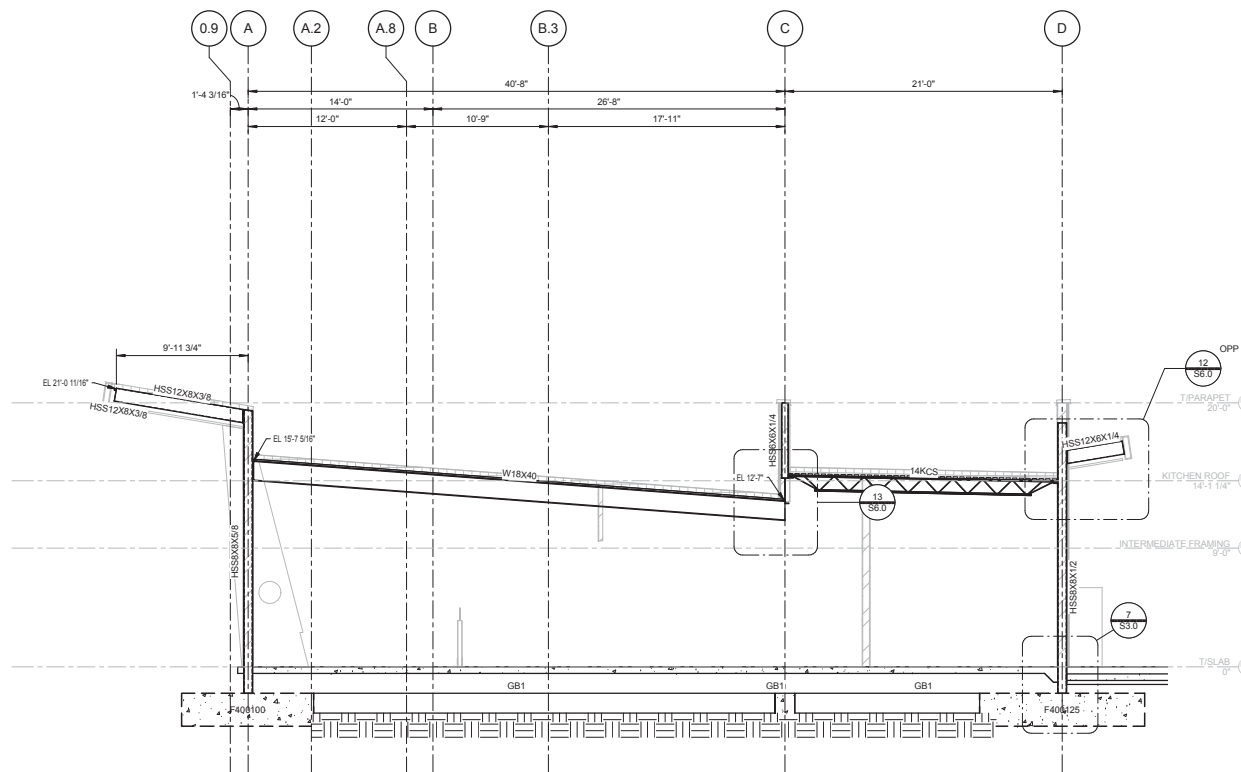
NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

2 ELEV LINE 6
SCALE: 3/16" = 1'-0"



NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

3 ELEV LINE 5
SCALE: 3/16" = 1'-0"



NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

4 ELEV LINE 4
SCALE: 3/16" = 1'-0"

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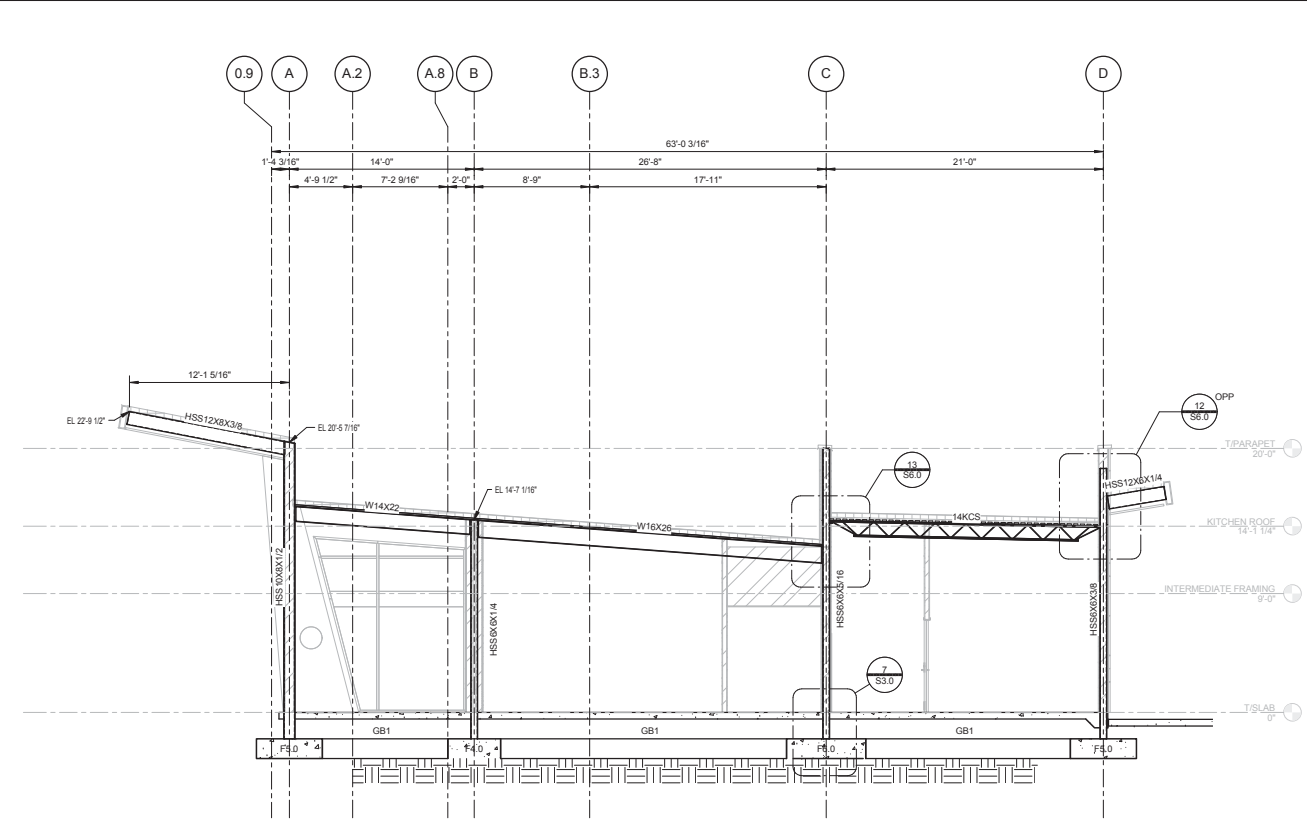
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BUILDING SECTIONS &
ELEVATIONS

Sheet Number:
S2.0

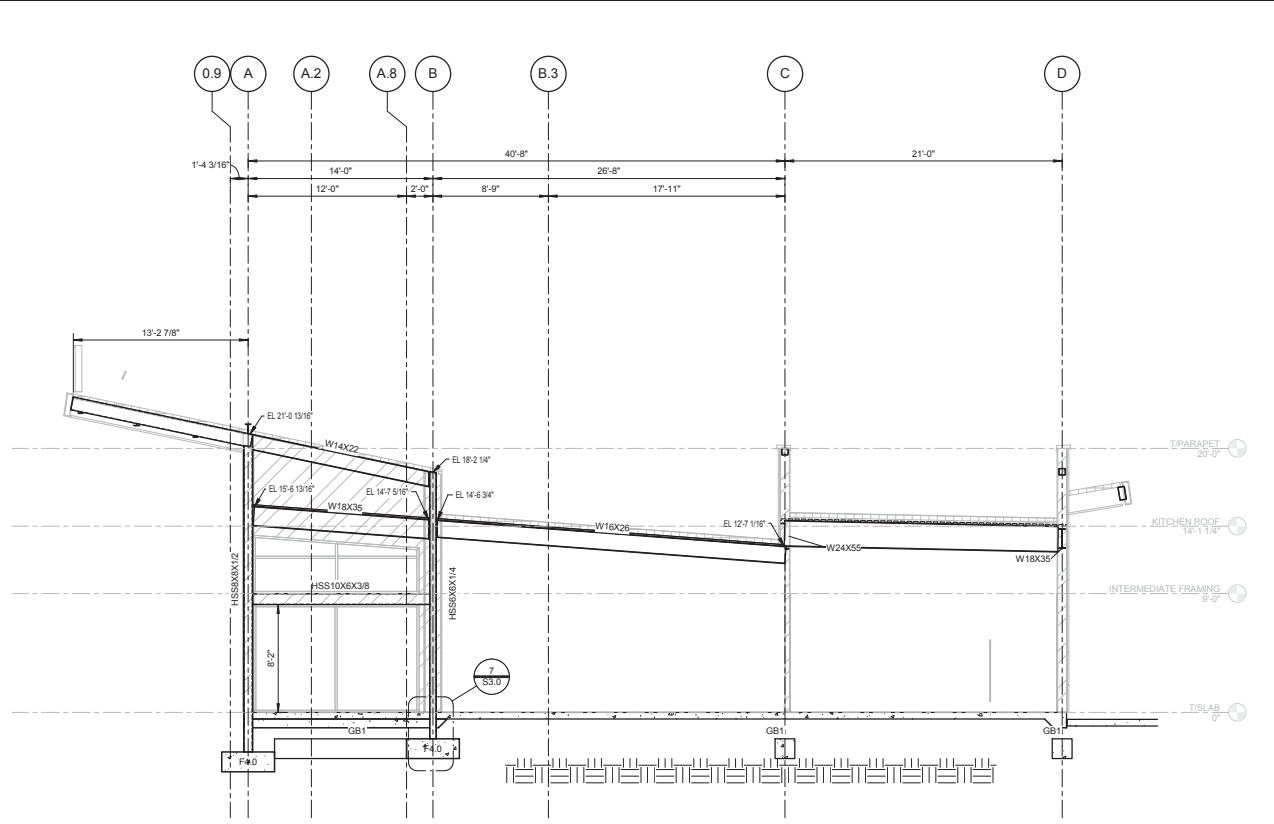
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916 W 5th Street
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Ph: (704) 547-9035 Fax: (704) 547-9836
email: mvm@mcveighmangum.com
Eng. of Record: Timothy Moore, PE License No: 76126

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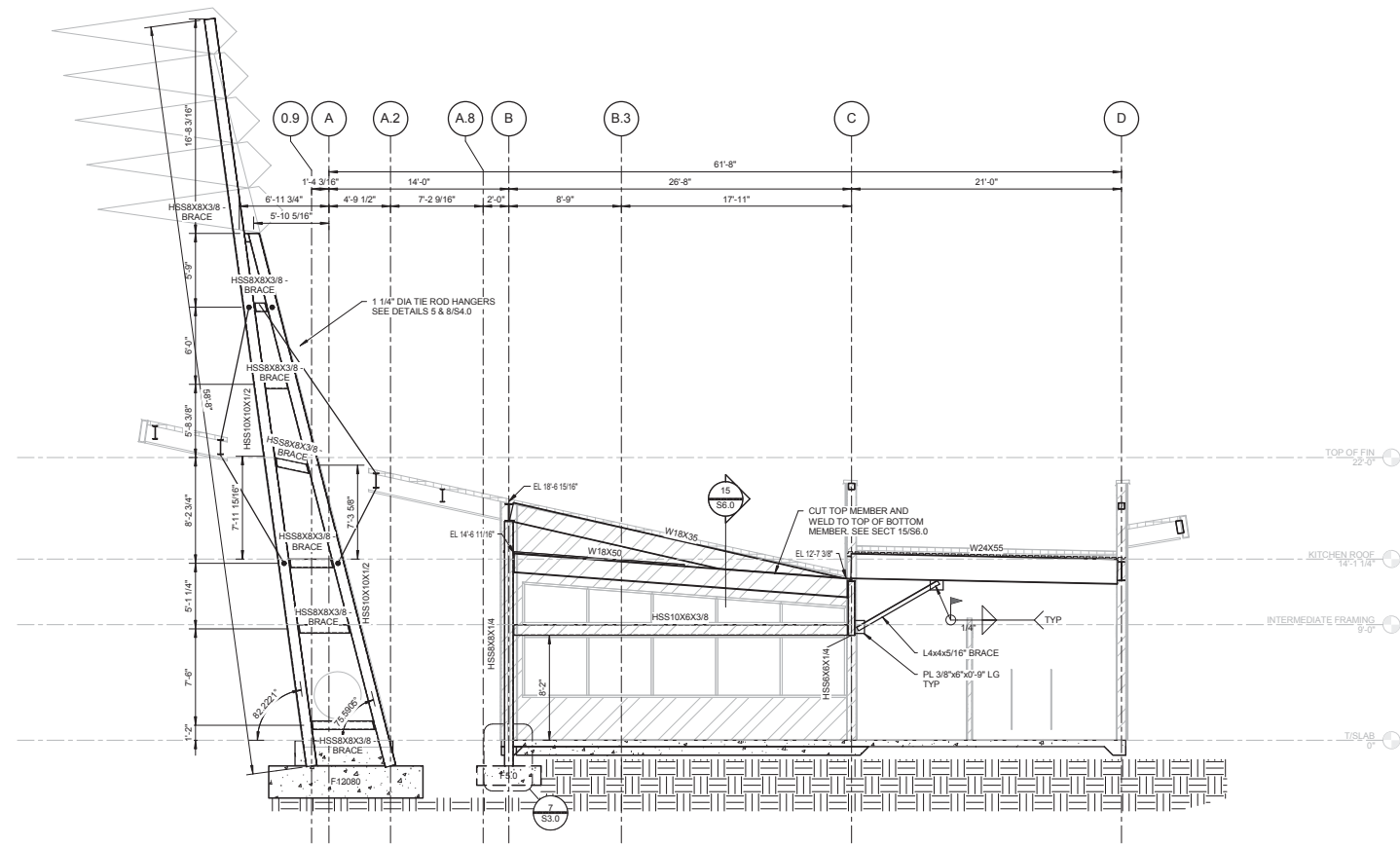
NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

1 ELEV LINE 3
 SCALE: 3/16" = 1'-0"



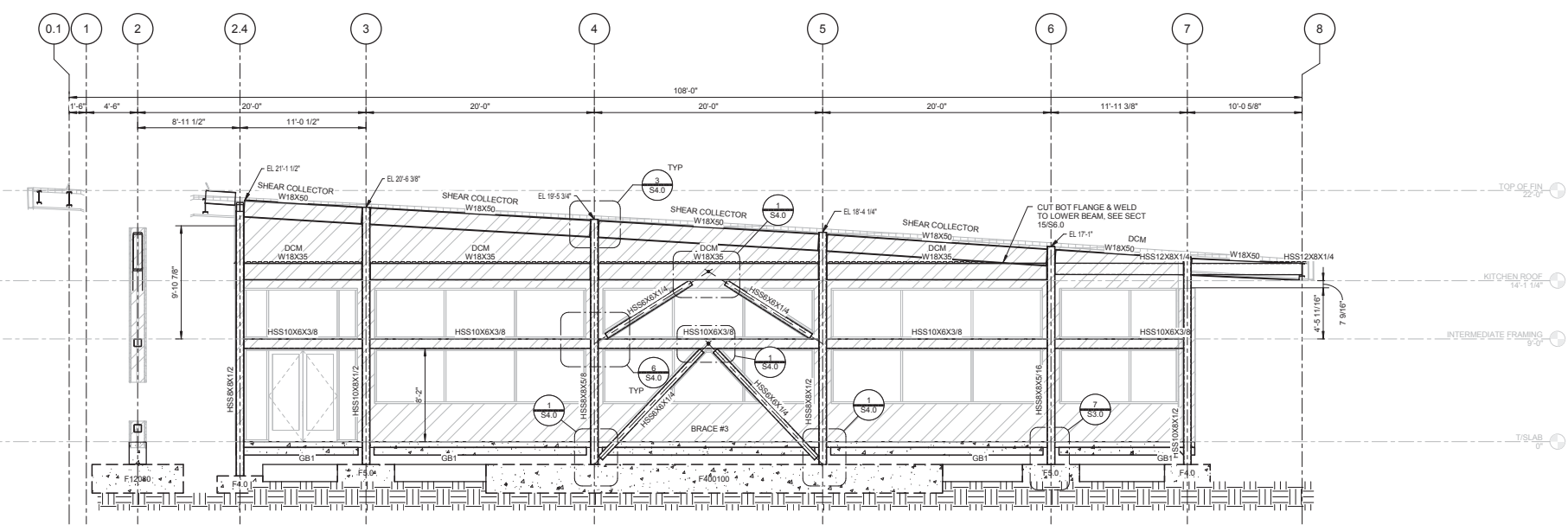
NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

2 ELEV LINE 2.4
 SCALE: 3/16" = 1'-0"



1
S2.2
MAIN SPIRE ELEVATION
SCALE: 3/16" = 1'-0"

NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.



2
S2.2
ELEVATION ALONG LINE A - (BRACE #3)
SCALE: 3/16" = 1'-0"

NOTE: DIMENSIONS SHOWN ARE TO CENTERLINE OF FRAMING, UNO.

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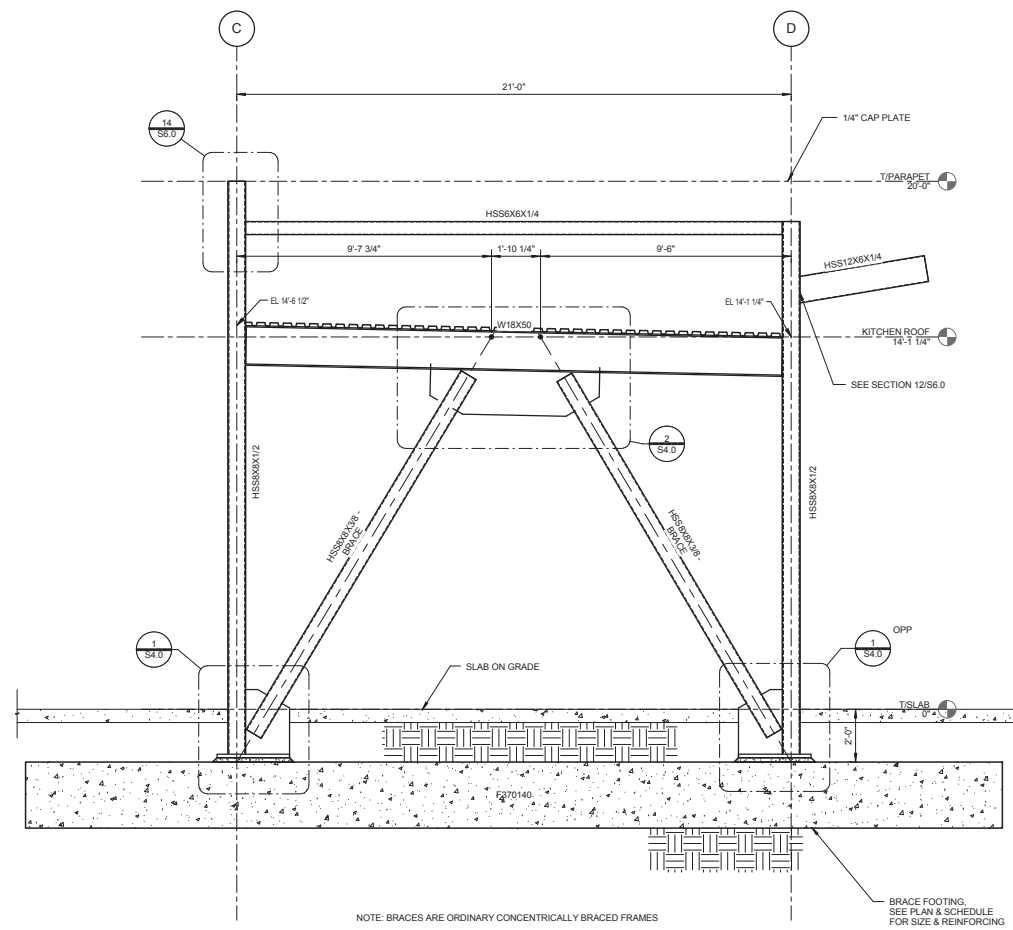
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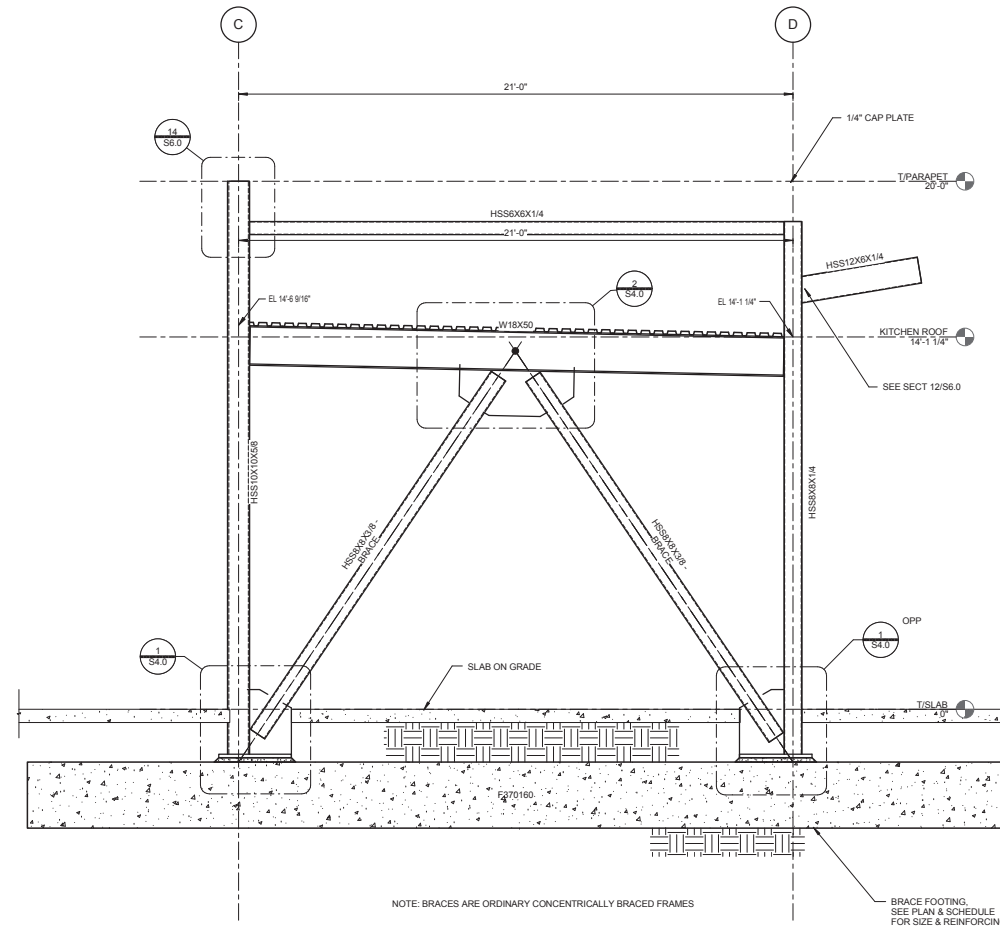
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BUILDING SECTIONS &
ELEVATIONS

Sheet Number:
S2.2

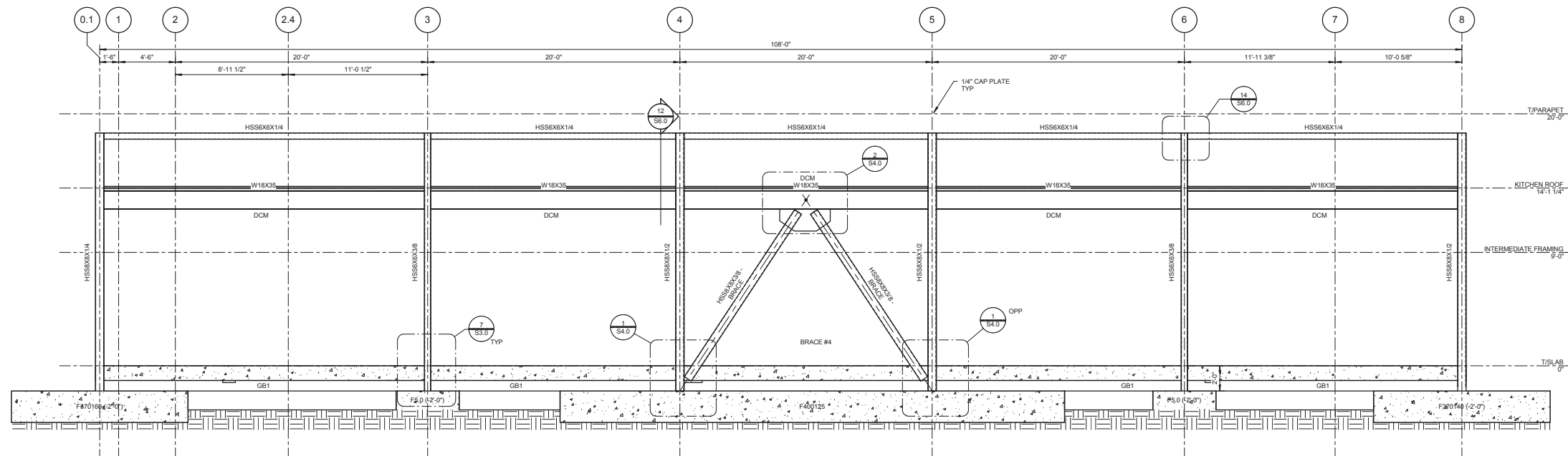
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ENGINEERING, INC.
916 W 5th Street
Charleston, North Carolina 28302
Ph: (704) 547-9035 Fax: (704) 547-9836
email: mvm@mcveighmangum.com
Eng. of Record: Timothy Moore, PE License No: 76426



1 BRACE FRAME #1 ELEVATION
SCALE: 3/8" = 1'-0"



2 BRACE FRAME #2 ELEVATION
SCALE: 3/8" = 1'-0"



4 ELEVATION ALONG D LINE - (BRACE #4)
SCALE: 1/4" = 1'-0"



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Designer:
WESLEY STEPHENS

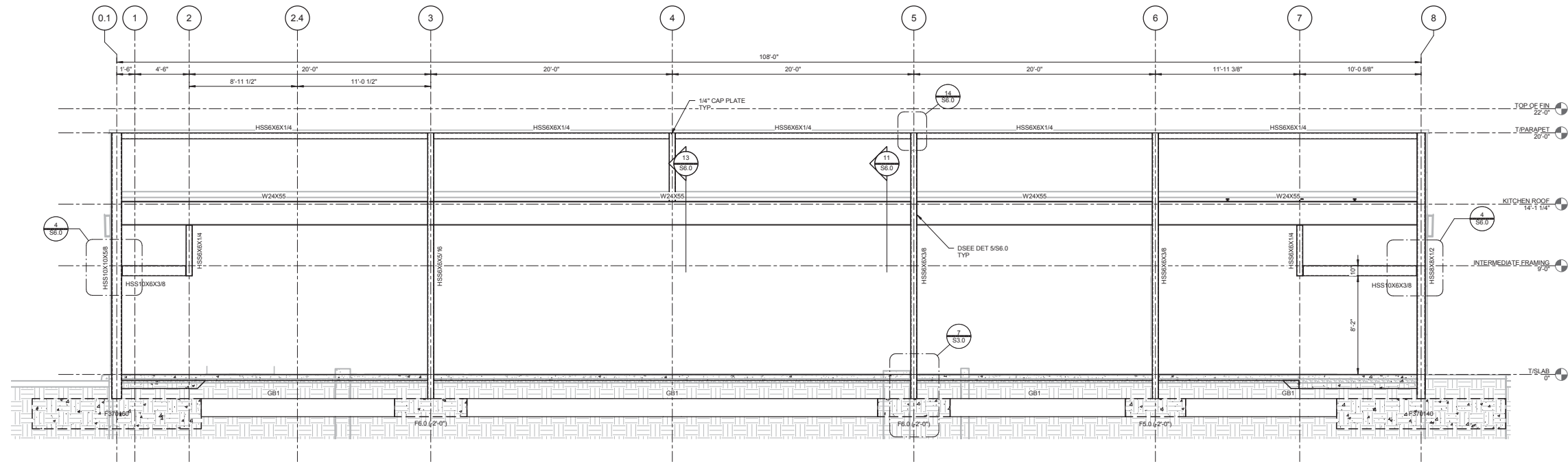
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1 ELEVATION ALONG C-LINE
SCALE: 1/4" = 1'-0"



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Principal in Charge:
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Project Manager:
WESLEY STEPHENS
Designer:
WESLEY STEPHENS

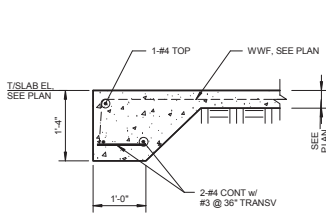
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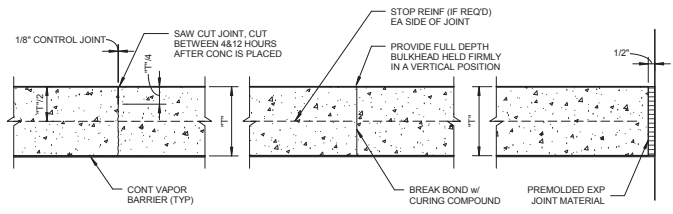
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S2.4





TYPICAL TURNDOWN

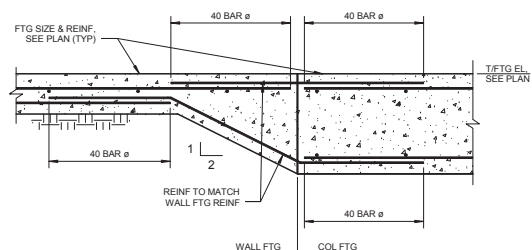
1 TYP TURNDOWN
SCALE: 3/4\"/>



CRACK CONTROL JOINT CONSTRUCTION JOINT EXPANSION JOINT AT WALL

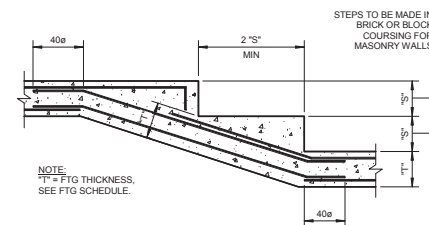
NOTES:
1) USE CONSTRUCTION JOINT INSTEAD OF CRACK CONTROL JOINT WHEREVER CONSTRUCTION IS STOPPED OR WHERE CALLED FOR ON PLAN.
2) PROVIDE CRUSHED STONE OR GRAVEL UNDER SLAB AS RECOMMENDED BY GEOTECHNICAL REPORT FOR THE PROJECT.

2 TYP SLAB ON GRADE JOINTS
SCALE: 3/4\"/>



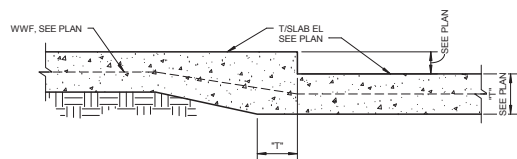
TYPICAL WALL FOOTING TO COLUMN FOOTING

3 WALL FTG TO COL FTG
SCALE: 3/4\"/>

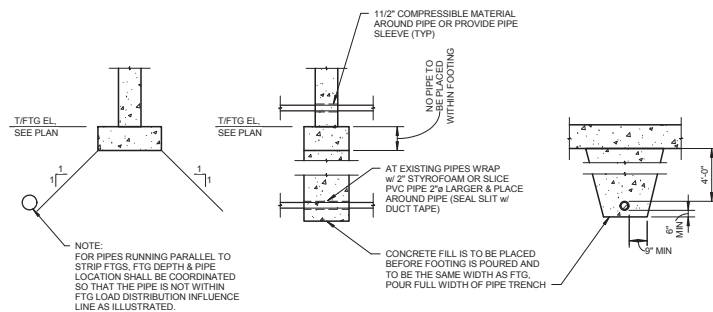


NOTE:
T = FTG THICKNESS.
SEE FTG SCHEDULE.

4 TYPICAL STEP IN FOOTING/GRADE BEAM
SCALE: 3/4\"/>

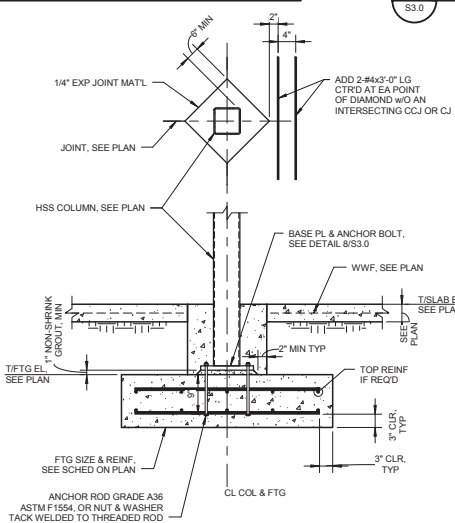


5 SLAB STEP DETAIL
SCALE: 3/4\"/>

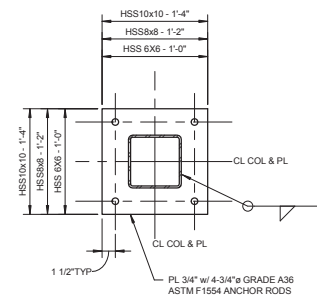


TYP PIPE PARALLEL TO WALL TYP PIPE PENETRATION THRU WALL

6 PIPE PENTERATION AT WALL FOOTING
SCALE: 3/16\"/>

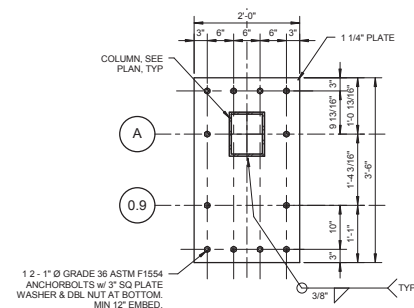


7 TYP TUBE COL FTG
SCALE: 3/4\"/>

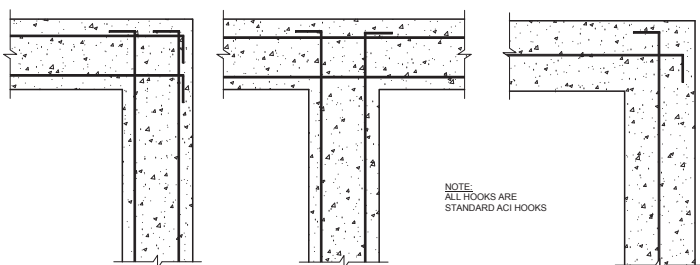


TUBE COLUMN BASE PLATE (NON-BRACED FRAME)

8 TUBE COLUMN BASE PL
SCALE: 1 1/2\"/>



9 TYPICAL TOWER BASE PLATE
SCALE: 3/4\"/>



TYPICAL CONCRETE WALL CORNERS & INTERSECTIONS

10 TYPICAL CONCRETE WALL CORNERS & INTERSECTIONS
SCALE: 3/4\"/>

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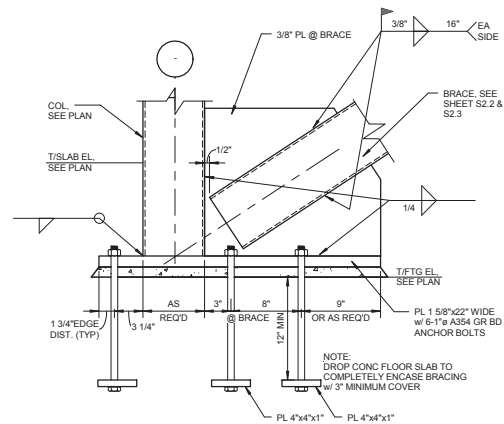
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FOUNDATION
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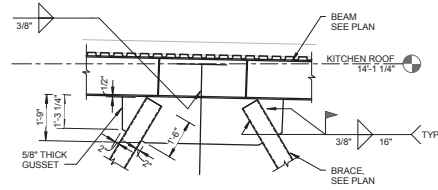
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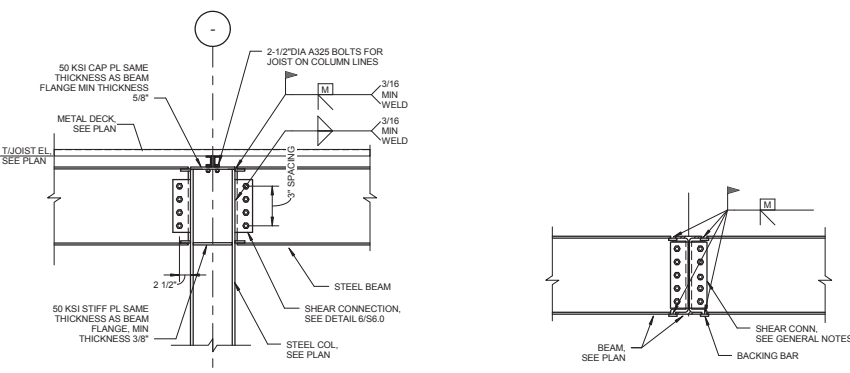
BASE PLATE AT DIAGONAL BRACE COLUMNS

1
S4.0 SCALE: 1 1/2" = 1'-0"



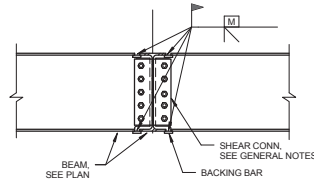
BRACE #1 - GUSSET

2
S4.0 SCALE: 3/8" = 1'-0"



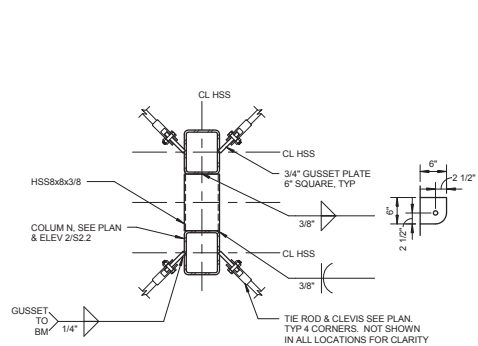
TYP COLUMN MOMENT CONNECTION

3
S4.0 SCALE: 3/4" = 1'-0"



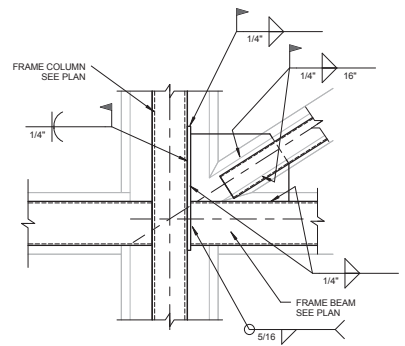
TYP BEAM TO BEAM MOMENT CONN

4
S4.0 SCALE: 3/4" = 1'-0"



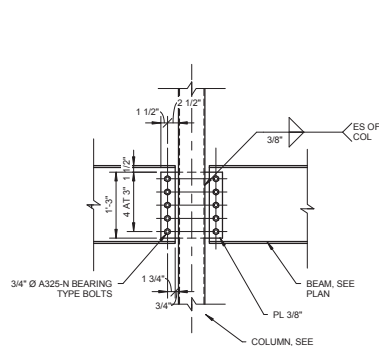
TOWER BRACE CONNECTION

5
S4.0 SCALE: NTS



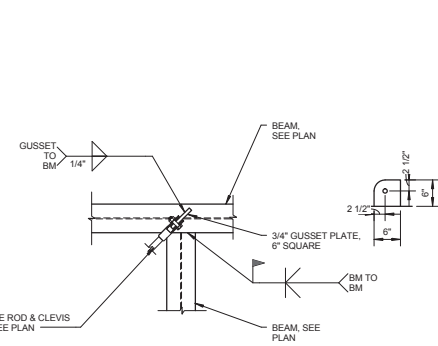
INTERMEDIATE BRACE CONNECTION

6
S4.0 SCALE: 3/4" = 1'-0"



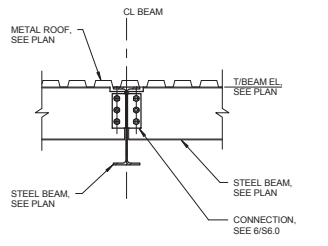
TYPICAL DCM CONNECTION

7
S4.0 SCALE: 3/4" = 1'-0"



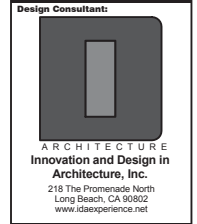
TIE ROD TO BEAM CONNECTION

8
S4.0 SCALE: 3/4" = 1'-0"



TYPICAL ROOF BEAM CONNECTION

9
S4.0 SCALE: 3/4" = 1'-0"



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